

# STORMWATER DEVELOPMENT MANUAL

PREPARED FOR:

## CITY OF HUNTINGTON

September 2005



**CITY OF HUNTINGTON  
STORMWATER DEVELOPMENT MANUAL**

**TABLE OF CONTENTS**

PART		Tab
1.0.	INTRODUCTION	1
1.1	Purpose of the Stormwater Development Standards	
1.2	Stormwater Management Board Description	
1.3	Stormwater Related Ordinances	
1.3.1	Erosion and Sediment Control	
1.3.2	Post-Construction Stormwater Runoff Control	
2.0	STORMWATER CONSTRUCTION PERMIT PROCESS	2
2.1	Review of Stormwater Development Standards	
2.2	Kickoff Meeting	
2.3	Project Development	
2.4	Permit Application	
2.5	Maintenance Agreement	
2.6	Review for Compliance	
2.7	Submittal Response	
2.8	Issuance of a Construction Permit	
2.9	Inspection	
2.10	Project Completion Review	
3.0	PERMIT APPLICATION COMPONENTS	3
3.1	Introduction	
3.2	Project Information	
3.2.1	Project Narrative	
3.2.2	Vicinity Map	
3.2.3	General Plan Requirements	
3.2.4	Existing Project Site Layout	
3.2.5	Final Project Site Layout	
3.2.6	Grading Plan	
3.2.7	Drainage Plan	
3.2.8	Erosion and Sediment Control Plan	
3.2.9	Post-Construction Stormwater Management Plan	
3.2.10	Technical Report	
3.2.11	Requirements for Category 3 Projects	
3.3	Permits for Construction in the Floodway	
4.0	HYDROLOGY	4
4.1	Introduction	
4.2	Calculating the Peak Discharge	
4.3	Design Storm Frequencies	
4.3.1	Minor Conveyance Systems	

4.3.2	Major Conveyance Systems	
4.3.3	Detention and Retention Facilities	
5.0	STORMWATER CONVEYANCE	5
5.1	Introduction	
5.2	Hydraulic Capacity	
5.3	Storm Sewers	
5.3.1	Sewer Pipe	
5.3.2	Structures	
5.3.3	Joints, Fittings and Appurtenances	
5.3.4	Construction Requirements	
5.4	Open Channels	
5.4.1	Channel Cross-Section and Grade	
5.4.2	Side Slopes	
5.4.3	Peak Flows	
5.4.4	Channel Stability	
5.4.5	Drainage of Waterways	
5.4.6	Establishment of New Regulated Drain	
5.4.7	Appurtenant Structures	
5.4.8	Disposition of Spoils	
5.4.9	Materials	
6.0	EROSION AND SEDIMENT CONTROL	6
6.1	Introduction	
6.1.1	Background and Purpose	
6.1.2	Design Principles	
6.2	Details on Specific Practices	
6.2.1	Temporary Sediment Basin	
6.2.2	Rock Check Dam	
6.2.3	Temporary Stone Construction Entrance	
6.2.4	Silt Fence	
6.2.5	Straw Dam	
6.2.6	Fabric Drop Inlet Protection	
6.2.7	Sandbag Curb Inlet Protection	
6.2.8	Riprap Chute Outfall Protection	
7.0	POST-CONSTRUCTION STORMWATER QUALITY CONTROL	7
7.1	Introduction	
7.1.1	Background and Purpose	
7.1.2	Stormwater Quality Control Requirements	
7.1.3	Water Quality Volume (WQv)	
7.1.4	Structural Best Management Practices	
7.2	Pre-Approved BMPs	
7.2.1	Stormwater Ponds	
7.2.2	Detention Basin	
7.2.3	Stormwater Wetlands	

- 7.2.4 Bioretention
- 7.2.5 Sand Filters
- 7.2.6 Dry Water Quality Swales
- 7.2.7 Biofilters
- 7.2.8 Catch Basin Inserts
- 7.2.9 Other Pre-Approved BMPs

APPENDIX A – Ordinances	A
Erosion and Sediment Control	
Control for Post-Construction Stormwater Runoff	
APPENDIX B – Permit Forms	B
Permit Application	
Submittal Package Checklist for Category 1 Projects	
Submittal Package Checklist for Category 2 Projects	
Submittal Package Checklist for Category 3 Projects	
Maintenance Agreement	
Submittal Response	
Permit Approval	
Inspection Logs	
APPENDIX C – Stormwater Conveyance Details	C
Structures	
Castings	
Bedding and Backfilling	
Open Channels	
APPENDIX D – Erosion and Sediment Control Details	D
Temporary Sediment Basin	
Rock Check Dam	
Temporary Stone Construction Entrance	
Silt Fence	
Straw Dam	
Fabric Drop Inlet Protection	
Sandbag Curb Inlet Protection	
Riprap Chute Outfall Protection	
APPENDIX E – Post-Construction Stormwater Runoff Control Details	E
Stormwater Ponds	
Dry Detention Basin	
Stormwater Wetlands	
Bioretention	
Sand Filters	
Water Quality Swales	
Biofilters	

## **1.0 INTRODUCTION**

### **1.1 Purpose of the Stormwater Development Standards**

It is recognized that smaller streams and drainage channels serving the City of Huntington may not have sufficient capacity to receive and convey additional stormwater runoff, as a result of development. Development is the changing of open or agricultural land to urban uses. It is further recognized that deposits of sediment from development, during and after, construction can reduce capacities of storm sewers and drainage systems and result in damages to receiving lakes and streams. This Stormwater Development Manual was developed to ensure the proper treatment, pollution prevention, storage, and conveyance of stormwater within the City of Huntington, as required by the EPA's Phase II Final Rule and the State of Indiana's 327-IAC 15-13 and 327 IAC 15-5.

EPA's Phase II Final Rule was published in December 1999. The objectives of the Phase II regulations are to reduce the discharge of pollutants to the "maximum extent practicable" and to protect the quality of the Nation's waters. Included in this rule are permit requirements for designated small municipalities and certain facilities that maintain control of a separate storm sewer system. The City of Huntington is one of these municipalities. They are referred to as a Municipal Separate Storm Sewer System (MS4) community.

The intent of this Stormwater Development Manual is to create a development process that is easily understood and consistently applied for all stormwater projects and improvements within the City of Huntington. The Huntington City Engineering Department has coordinated the process to be used, the implementation of the rules and procedures, and the intent of the regulations and ordinances all of which manage the development of stormwater improvements. The developer, engineering consultants, architects, and municipality can be guided through the process so that each stakeholder knows the permit requirements, standards, and details associated with the design criteria.

The Stormwater Development Manual is comprised of the following sections:

- Stormwater Construction Permit Process
- Stormwater Construction Permit Application Components
- Hydrology
- Stormwater Conveyance
- Erosion and Sediment Control
- Post-Construction Stormwater Control

## **1.2 Stormwater Management Board Description**

The Huntington Stormwater Management Board was formed in 1996. The Board is controlled by three appointed Board Members.

The three main objectives of the board are as follows:

- To allow the local government to respond to stormwater problems,
- To establish a plan for collecting and disposing of stormwater, and
- To create alternatives by which to finance the handling of stormwater.

## **1.3 Stormwater Related Ordinances**

The following sections describe the stormwater related ordinances for the City of Huntington. The following ordinances can be found in **Appendix A**, Stormwater Related Ordinances.

### **1.3.1 Erosion and Sediment Control**

The purpose of the Erosion and Sediment Control Ordinance is to minimize the amount of sediment and other pollutants resulting from soil erosion during land disturbing activities. These pollutants are often transported off-site to adjacent public or private lands including ditches, streams, lakes, wetlands, reservoirs, storm sewers and MS4 conveyances without proper erosion and sediment control.

### **1.3.2 Post-Construction Stormwater Runoff Control**

The purpose of the Post-Construction Stormwater Runoff Control Ordinance is to establish minimum stormwater management requirements during the design process in order to reduce, non-point source pollution and the rate and volume of surface water run-off once the construction phase has been completed.

## 2.0 STORMWATER CONSTRUCTION PERMIT PROCESS

The following sections describe the stormwater construction permit process for the City of Huntington.

### 2.1 Review of Stormwater Development Standards

The applicant should review this manual and familiarize themselves with the permit process, the forms, specifications, construction details, etc. It is also advised that the applicant review the Erosion and Sediment Control Ordinance and the Post-Construction Stormwater Run-off Control Ordinance. Once the applicant is familiar with all three, the manual and ordinances may then be used as reference documents.

### 2.2 Kickoff Meeting

The applicant shall request a meeting with a City representative to review permit requirements. The applicant is encouraged to submit a sketch or conceptual plan of the proposed stormwater improvements with the initial meeting request. There are no fees associated with this submittal.

### 2.3 Project Development

The applicant shall incorporate stormwater improvements (stormwater quantity control and quality control) into the design of their project and submit their proposed development for review to the City. Appropriate stormwater control measures are discussed in greater detail in Chapters 6 and 7.

### 2.4 Permit Application

The applicant shall submit to the City Engineering Department at 300 Cherry Street, Huntington, Indiana 46750 the following items:

- The Stormwater Construction Permit Application form;
- Submittal Package Checklist;
- Plans;
- Specifications; and
- Design documentation.

The Stormwater Construction Permit Application form and the Submittal Package Checklist can be found in **Appendix B**, Permit Forms. Please note that there are three separate categories of applications. They are: **Category 1**, land disturbing activities covering one or more acres, **Category 2**, single family residential development consisting of four or fewer lots or a single-family residential strip development where the developer offers for sale or lease without land improvements, and the project is not part of a larger

common plan of development or sale, and **Category 3**, land disturbing activities for individual lots of any size within a permitted project that is over one acre. There is one common permit application, but there are three separate checklists. A filing fee will be required in order to initiate the application review process. The components of the permit application are discussed further in Chapter 3.

## **2.5 Maintenance Agreement**

Maintenance of all stormwater management facilities will be ensured through the establishment of a formal maintenance agreement. The applicant shall submit, along with the permit application, a completed Maintenance Agreement form. The Maintenance Agreement form can be found in **Appendix B**, Permit Forms.

## **2.6 Review for Compliance**

Upon receipt of the Stormwater Construction Permit Application and attachments, the proposed development will be reviewed by a representative of Huntington City Engineering Department or its designee. The proposed development will be reviewed to ensure that the requirements of this Stormwater Development Manual have been incorporated into the project.

## **2.7 Submittal Response**

Once the City Engineering Department has reviewed the proposed development and determined whether or not it is in full compliance with the requirements of this manual, the City Engineering Department will issue a Submittal Response to the applicant. The Submittal Response form can be found in **Appendix B**, Permit Forms. The response will indicate if the required information has been provided for the proposed development to be reviewed or if additional information is required before the comprehensive review process can begin. If the required information has been provided, the proposed development is ready for the comprehensive review process which results in a construction permit.

## **2.8 Issuance of a Construction Permit**

The proposed development will be reviewed against the design criteria set forth in this manual. Upon completion of the comprehensive review process, the project will be issued a Stormwater Construction Permit. The Construction Permit Form can be found in **Appendix B**, Permit Forms. Should the applicant receive a stormwater construction permit, he/she must then submit the required Notice of Intent (NOI) to IDEM. (Individual lots within a permitted project do not have to submit a NOI.)



## **2.9 Inspection**

### Construction Inspections

The City will conduct regular inspections of construction sites during land-disturbing activities, in accordance with the Erosion and Sediment Control Ordinance, to ensure compliance with a Stormwater Construction Permit.

All inspections will be documented in writing. These reports will contain the following information when applicable:

- The date and location of the inspection;
- Whether construction is in compliance with the approved stormwater management plan;
- Variations from the approved construction specifications; and
- Any violations that exist.

### Post-Construction Inspections

Post-construction stormwater runoff inspections may also be conducted by the City, in accordance with the requirements of the Post-Construction Stormwater Runoff Control Ordinance, to ensure compliance with a Stormwater Construction Permit.

All inspections will be documented in writing. These reports will contain the following information when applicable:

- The date and location of the inspection;
- Whether or not the stormwater BMP is being properly maintained
- Whether or not the stormwater BMP is operating properly
- Any violations that exist.

## **2.10 Project Completion Review**

When the construction phase of a project has been completed, the person holding the permit shall request, in writing, City approval of the permanent erosion and sediment control measures constructed. The City will then evaluate the adequacy of the constructed control measures.

In the event that the permanent erosion control measures are approved, any surety bonds and/or letters of credit shall be released. However, all maintenance responsibilities shall remain with the person owning the land.

In the event that the erosion control measures are not approved because they are not in accordance with the Erosion and Sediment Control Plan, the City shall notify, in writing, the person holding the permit of unacceptable features.

## 3.0 PERMIT APPLICATION COMPONENTS

### 3.1 Introduction

This chapter describes the permit application components for the submittal of a stormwater permit application for proposed developments within the City of Huntington.

### 3.2 Project Information

The following information for development and redevelopment on real estate located within the MS4 Area shall be submitted to the Huntington City Engineering Department at the time of application. This includes data certified by an Indiana licensed professional engineer or land surveyor engaged in storm drainage design. The following information requirements apply to: **Category 1** - land disturbing activities covering one or more acres, **Category 2** - single family residential development consisting of four or fewer lots or a single-family residential strip development where the developer offers for sale or lease without land improvements, and the project is not part of a larger common plan of development or sale and **Category 3** - Land disturbing activities for individual lots of any size within a permitted project that is over one acre.

#### 3.2.1 Project Narrative (Category 1 and Category 2)

The applicant shall provide a project narrative which includes at a minimum the following:

- The scope of the project;
- The purpose of the project;
- A legal description of the project location;
- Existing soil conditions;
- The construction sequence;
- A project site map or plat;
- The Hydrologic Unit Code; and
- Identification of other State of Federal Water Quality Permits required.

### **3.2.2 Vicinity Map (Category 1 and Category 2)**

The applicant shall provide a map illustrating the location and vicinity with their submittal for a stormwater construction permit. The vicinity map shall include roads, railroads, water features and other relevant landmarks.

### **3.2.3 General Plan Requirements (Category 1 and Category 2)**

The plans shall include, in general, the following:

- North arrows, graphic and written scale, legend for symbols;
- Location and elevation of property benchmark;
- Professional seal, signature, and date; and
- Table of revisions.

### **3.2.4 Existing Project Site Layout (Category 1 and Category 2)**

The existing project site layout shall include the existing site conditions. This project site layout will require much more site specific detail than the vicinity map described above. The project site map shall contain, at a minimum, the following:

- Site location;
- Site boundaries;
- Immediately adjacent sites;
- Lakes, streams, channels, ditches, wetlands and other water courses on or adjacent to the site;
- 100-year floodplains, floodway fringes and floodways;
- Location of the predominant soil types. Soil types may be determined by the SCS County Soil Survey, by an equivalent publication, or as determined by a certified professional soil scientist;
- Location and delineation of vegetative cover such as grass, weeds, brush, and trees, and any vegetation areas that will not be disturbed during construction;

- Location of natural drainage patterns on and immediately adjacent to the site, and the location and approximate dimensions of any structural drainage systems;
- Locations and approximate dimensions of existing utilities, structures, roads, highways and paving;
- Site and adjacent topography, both existing and planned, at a minimum of 2 foot contour interval to indicate drainage patterns.

### **3.2.5 Final Project Site Layout (Category 1 and Category 2)**

The final project site layout shall include the existing site conditions. This project site layout will require much more site specific detail than the vicinity map described above. The project site map shall contain, at a minimum, the following:

- The location of all proposed site improvements including roads, utilities, lot delineation, structures and common areas.

### **3.2.6 Grading Plan (Category 1 and Category 2)**

The grading plan shall include, at a minimum:

- A delineation of proposed land disturbing activities, including those off-site;
- Soil stockpiles and borrow areas;
- Information regarding off-site borrow, stockpile and disposal areas that are under the control of the project site owner; and
- Existing and proposed topographic information.

### **3.2.7 Drainage Plan (Category 1 and Category 2)**

The drainage plan shall include, at a minimum, the following:

- The peak discharge of a 10-year storm event, one for pre-development and one for post-development;

- The location, size, dimension, slope, and the direction of flow of all natural and structural stormwater drainage systems. These shall be prepared as both plan and profile views.
- When applicable, areas where point source stormwater discharges have the potential to enter ground water.
- All stormwater discharge points in which stormwater will leave the site. In addition, if the discharge point is to a receiving stream, the stream must be labeled by name. If the discharge point is to a conveyance system, the name of the receiving stream in which the conveyance system discharges must be identified.
- The location, size and dimensions of site features used for stormwater management. Examples of such features include detention and retention ponds.
- Off-site bypass flow route for detention and retention ponds, if applicable.
- Bottom pond slope for detention and retention ponds, if applicable.
- Flood protection grades for detention and retention ponds, if applicable.

### **3.2.8 Erosion and Sediment Control Plan (Category 1 and Category 2)**

The Erosion and Sediment Control Plan shall clearly portray the methods and means whereby erosion and sediment control measures are implemented. The plan shall include, at a minimum, the following site development information.

- A description of potential pollutant sources associated with construction activities that may be expected to add a significant amount of pollution to the developments stormwater discharges;
- The location, dimensions, detailed specifications and construction details of all temporary and permanent stormwater quality measures;

- Temporary and permanent stabilization measures including: the sequence of implementation; specifications and application rates for soil amendments and seed mixtures; and the type and application rate for anchored mulch;
- The construction sequence describing the relationship between implementation of stormwater quality measures and the stages of construction activities; and
- A self monitoring program including procedures and an on-going maintenance plan for each stormwater quality measure.

The person, or persons, responsible for the installation and maintenance of erosion and sediment control practices, their business address, and daytime phone number shall be included. This person must be identified prior to the commencement of any land disturbing activities.

### **3.2.9 Post-Construction Stormwater Management Plan (Category 1 only)**

The Post-Construction Stormwater Management Plan shall clearly portray the methods and means whereby stormwater quality measures are implemented. The plan shall include, at a minimum, the following information.

- A description of potential pollutant sources associated with the proposed land use that may be expected to add a significant amount of pollution to the developments stormwater discharges.
- The location, dimensions, detailed specifications, construction details, and sequence of installation of all post-construction stormwater quality measures. These measures must remove or minimize pollutants from stormwater run-off and prevent or minimize adverse impacts to stream and riparian habitats.
- A self monitoring program including procedures and an on-going maintenance plan for each stormwater quality measure.

### **3.2.10 Technical Report (Category 1 and Category 2)**

The applicant shall submit a technical report providing all calculations.

#### **3.2.10.1 Existing and Proposed Stormwater Runoff Calculations**

The existing and proposed stormwater runoff calculations must include, at a minimum, the following:

- Drainage area computations;
- Weighted curve number or run-off coefficient computations; and
- Time of concentration computations.

#### **3.2.10.2 Closed Conduit and Open Channel Design Calculations**

The closed conduit and open channel design calculations must include, at a minimum, the following:

- The size of pipe or channel shown as a cross-section;
- The pipe or channel invert slope as a percentage and as invert elevations;
- Material and roughness coefficient;
- Flow velocities in feet per second;
- Pipe length, type, and class;
- Design capacity in cubic feet per second;
- Design run-off of component sheds; and
- Inlet spacing calculations.

### **3.2.10.3 Detention and Retention Pond Calculations**

The detention and retention pond calculations must include, at a minimum, the following:

- Shed maps, both pre-development and post-development;
- Time of concentration, both pre-development and post-development;
- Overland flow route to pond;
- Peak design discharge;
- Time to peak discharge;
- Total design storm runoff;
- Inflow hydrograph;
- Spillway peak outflow rate;
- Emergency spillway capacity calculations;
- Off-site bypass flow route;
- Bottom pond slope; and
- Flood protection grades on plat.

### **3.2.11 Requirements for Category 3 Projects**

The following items must be submitted for a Category 3 activity:

- Post-Construction Stormwater Management Plan from original Category 1 project;
- General layout of lot including position of building;
- North arrow, graphic and written scale, date; and
- Stable construction site access and appropriate perimeter erosion and sediment control measures.



### **3.3 Permits for Construction in the Floodway**

It is a requirement that the Department of Natural Resources (DNR) approve all construction activities being proposed in a floodway, as well as all works for flood control. This includes bridges, dams, levees, floodwalls, wharves, piers, booms, weirs, bulkheads, jetties, groins, excavations, fills, or deposits of any kind, utility lines, or any other building, structure, or obstruction. This also includes any ditch work (new construction, deepening or modification) within one-half mile of a public freshwater lake of 10 acres or more.

The approval of the DNR, in writing, must be obtained before land disturbing activities can begin. Applications for approval shall be submitted to:

Indiana Department of Natural Resources  
Division of Water  
402 West Washington St., Room W264  
Indianapolis, IN 46204

All applications should be sent to the DNR on their standard application form. Similar to this manual's filing requirements, the DNR's application form shall be accompanied by plans, profiles, specifications, and other data necessary for the DNR to determine the effect of the proposed development on the floodway.

It should be noted that an application made to and approved by the DNR does not in any way relieve the owner of the necessity of securing easements or other property rights, and permits and/or approvals from affected property owners and local, State, and Federal agencies.

The engineering staff of the DNR Division of Water is available to discuss and offer suggestions regarding requirements in the design of structures in floodways. High water marks have been set on many of the streams in the state, and information is available from the Division of Water on actual and/or potential flooding. Information regarding benchmarks set to Mean Sea Level Datum, General Adjustment of 1929, is available from the Division of Water, Surveying, and Mapping Section.

## 4.0 HYDROLOGY

### 4.1 Introduction

This chapter describes the policies and procedures which must be applied during a hydrologic analysis performed within the City of Huntington.

### 4.2 Calculating the Peak Discharge

Runoff quantities shall be computed for the area of the parcel under development, including runoff that flows to the proposed development's site from the site's surrounding watershed. The calculation shall be done based on pre-development conditions and again with the proposed post-development conditions. The peak discharge which is generated as a result of a given rainfall intensity may be calculated as follows:

#### **Areas up to and Including 200 Acres**

For areas up to and including 200 acres, the Rational Method may be used, to determine the peak discharge rate

$$Q = CiA$$

Where:

C = runoff coefficient, representing the characteristics of the drainage area and defined as the ratio of runoff to rainfall.

i = average intensity of rainfall in inches per hour for a duration equal to the time of concentration ( $t_c$ ) for a selected rainfall frequency.

A = tributary drainage area in acres.

The following tables provide guidance to selection of the runoff coefficient "C" for different types of surface and soil characteristics. The composite "C" value used for a given drainage area with various surface types shall be the weighted average for the total area calculated from a breakdown of the individual areas having different surface types.

<b>TABLE 4-1</b>	
<b>Urban Runoff Coefficients</b>	
<b>Type of Surface</b>	<b>Runoff Coefficient "C"</b>
Asphalt	0.82
Concrete	0.85
Roof	0.85
Lawns (Sandy)	
Flat (0-2% slope)	0.07
Rolling (2-7% slope)	0.12
Steep (>7% slope)	0.17
Lawns (Clay)	
Flat (0-2% slope)	0.16
Rolling (2-7% slope)	0.21
Steep (>7% slope)	0.30

<b>TABLE 4-2</b>	
<b>Rural Runoff Coefficients</b>	
<b>Type of Surface</b>	<b>Runoff Coefficient "C"</b>
Woodland (Sandy)	
Flat (0-5% slope)	0.10
Rolling (5-10% slope)	0.25
Steep (>10% slope)	0.30
Woodland (Clay)	
Flat (0-5% slope)	0.30
Rolling (5-10% slope)	0.35
Steep (>10% slope)	0.50
Pasture (Sandy)	
Flat (0-5% slope)	0.10
Rolling (5-10% slope)	0.16
Steep (>10% slope)	0.22
Pasture (Clay)	
Flat (0-5% slope)	0.30
Rolling (5-10% slope)	0.36
Steep (>10% slope)	0.42
Cultivated (Sandy)	
Flat (0-5% slope)	0.30
Rolling (5-10% slope)	0.40
Steep (>10% slope)	0.52
Cultivated (Clay)	
Flat (0-5% slope)	0.50
Rolling (5-10% slope)	0.60
Steep (>10% slope)	0.72

<b>TABLE 4-3</b>				
<b>Runoff Coefficients "C" by Land Use and Typical Inlet Times</b>				
<b>LAND USE</b>	<b>RUNOFF COEFFICIENTS</b>			<b>INLET TIME (minutes)</b>
	<b>FLAT</b> (0-2% slope)	<b>ROLLING</b> (2-7% slope)	<b>STEEP</b> (>7% slope)	
Commercial (CBD)	0.75	0.83	0.91	5
Commercial (NHD)	0.54	0.60	0.66	5
Industrial	0.63	0.70	0.77	5-10
Garden Apartments	0.54	0.60	0.66	5-10
Churches	0.54	0.60	0.66	5-10
Schools	0.31	0.35	0.39	10-15
Semi-Detached Residence	0.45	0.50	0.55	10-15
Detached Residence	0.40	0.45	0.50	10-15
Quarter-Acre Lots	0.36	0.40	0.44	10-15
Half-Acre Lots	0.31	0.35	0.39	10-15
Parkland	0.18	0.20	0.22	To be determined

Interpolation, extrapolation, and adjustment for local conditions shall be based on engineering experience and judgment.

The coefficients of these tabulations are applicable to storms of 5- to 10-year frequencies. Coefficients for less frequent, higher intensity storms shall be modified as follows

<b>Return Period (years)</b>	<b>Multiply "C" by</b>
25	1.1
50	1.2
100	1.25

Rainfall intensity shall be determined from the rainfall frequency curves shown in Figure 4-1. The time of concentration (tc) to be used shall be the sum of the inlet time and flow time in the drainage facility from the most remote part of the drainage area to the point under consideration. The flow time may be estimated by the distance in feet divided by the velocity of flow in feet per second.

Inlet time is the combined time required for the runoff to reach the inlet of the storm sewer. It includes overland flow time through established surface drainage channels such as swales, ditches and sheet flow across such areas as lawns, fields, and other graded surfaces. An example computation is provided in Figure 4-2.

Figure 4-1: Rainfall Intensity-Duration-Frequency Curves  
Fort Wayne, Indiana 1911 - 1951

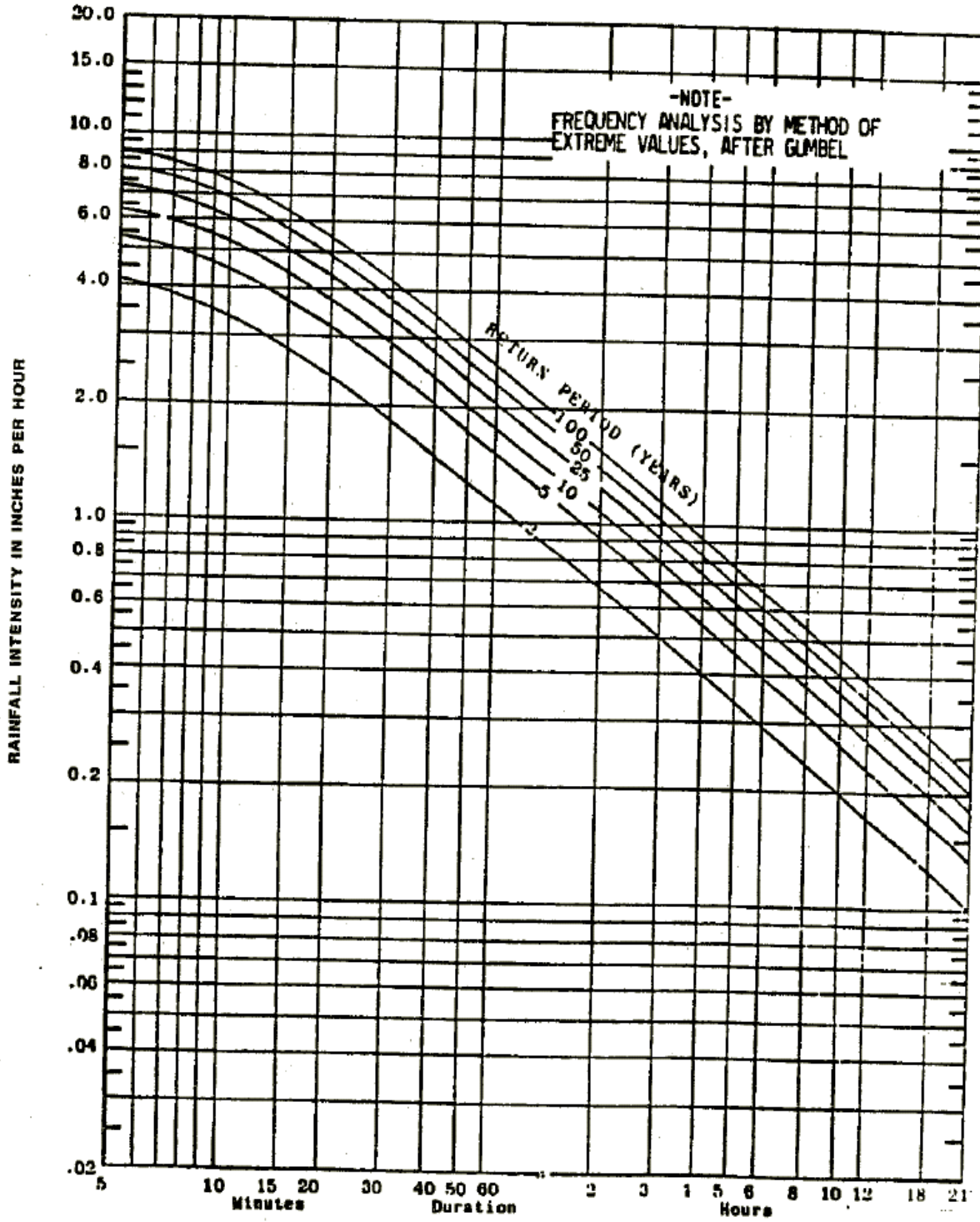
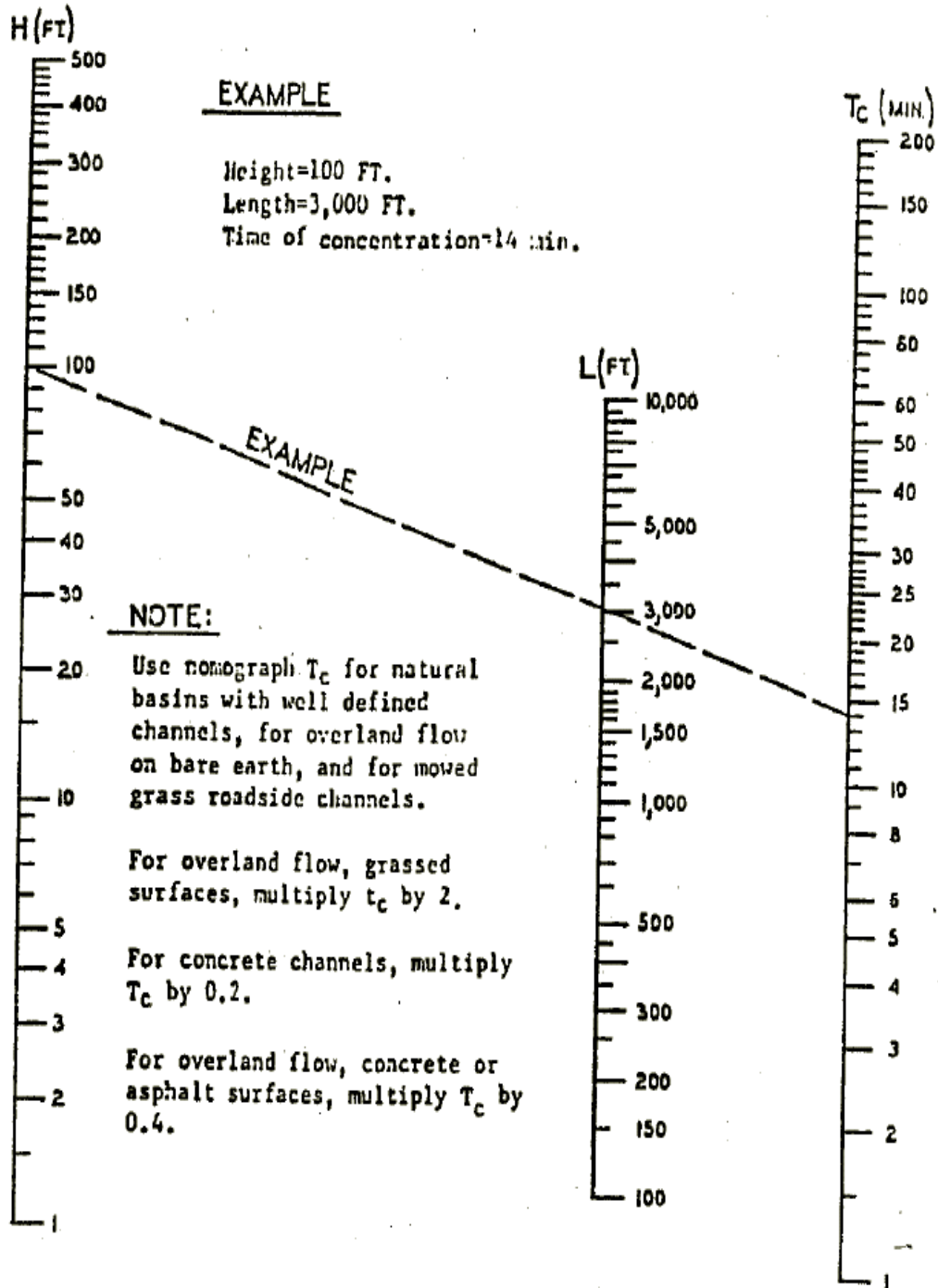


Figure 4-2: Nomograph for Determining Time of Concentration (developed from the Kirpich Equation)



## **Areas Over 200 Acres**

The peak discharge rate for areas in excess of 200 acres shall be determined by methods approved by the City Engineering Department. The procedures or methods used must receive prior approval. The TR-20 and TR-55 models are approved for appropriate use in analysis of the runoff and routing of stormwater.

### **4.3 Design Storm Frequencies**

Design storm frequencies shall be chosen based on the following criteria.

#### **4.3.1 Minor Conveyance Systems**

Minor conveyance system components such as inlets, catch basin inserts, street gutters, swales, sewers, and small channels which collect stormwater must accommodate peak runoff from a 10-year return period storm. Rainfall duration shall be less than or equal to the time of concentration for one hour, if the time of the concentration is less than or equal to one hour. A first quartile storm distribution shall be used for computer modeling.

#### **4.3.2 Major Conveyance Systems**

Major conveyance systems are defined as any drainage system carrying runoff from an area of one or more square miles and shall be designed in accordance with *Indiana Department of Natural Resources* standards.

#### **4.3.3 Detention and Retention Facilities**

Design of stormwater detention facilities shall be based on a return period of once in 100 years.

The release rate of stormwater from development, redevelopment, and new construction may not exceed the stormwater runoff from the land area in its present state of development. The developer must submit to the City detailed computations of runoff before and after development, redevelopment, or new construction which demonstrate that release rate of runoff will not be increased.

These computations must show that the peak runoff rate after development for the 100-year return period storm of critical duration must not exceed the 2-year return period pre-development peak runoff rate. The critical duration storm is that storm duration that requires the greatest detention storage.

In the event the natural downstream channel or storm sewer is inadequate to accommodate the release rate provided above, then the allowable release rate shall be reduced to that rate permitted by the capacity of the receiving downstream channel or storm sewer system and additional detention as determined by the City Engineering Department shall be required to store that portion of the runoff exceeding the capacity of the receiving sewers or waterways.

If more than one detention basin is involved in the development of the area upstream of the limiting restriction, the allowable release rate from any one detention basin shall be in direct proportion to the ratio of its drainage area to the drainage area of the entire watershed upstream of the restriction.



## 5.0 STORMWATER CONVEYANCE

### 5.1 Introduction

This chapter describes policies, design criteria and information for the construction of stormwater conveyances. Stormwater conveyance details are located in **Appendix C**, Stormwater Conveyance Details.

### 5.2 Hydraulic Capacity

The hydraulic capacity of all conveyances shall be determined using Manning's Equation:

$$v = (1.486/n) R^{2/3} S^{1/2}.$$

Where:

v = mean velocity in feet per second

R = the hydraulic radius in feet

S = the slope of the energy grade line in feet per foot

n = the roughness coefficient

The hydraulic radius, R, is defined as the cross-sectional area divided by the wetted flow surface or wetted perimeter.

The following table provides guidance to the selection of roughness coefficient (n) values for various materials. Roughness coefficient (n) values for materials can also be found in standard hydraulics texts and references.

**TABLE 5-1****Typical Values of Manning's (n) Roughness Coefficient**

<b>MATERIAL</b>	<b>MANNING'S (n)</b>	<b>DESIRABLE MAXIMUM VELOCITIES</b>
Closed Conduits		
Concrete	0.013	15 fps
Vitrified Clay	0.013	15 fps
Brick	0.015	15 fps
Cast Iron	0.013	15 fps
Circular Corrugated Metal, Annular Corrugations (2 2/3" x 1/2")		
Unpaved	0.024	7 fps
25% Paved	0.021	7 fps
50% Paved	0.018	7 fps
100% Paved	0.013	7 fps
C.C.M.P., Helical Corrugations, 2/3" x 1/2" Unpaved Corrugation		
12"	0.022	7 fps
18"	0.023	7 fps
24"	0.024	7 fps
36"	0.025	7 fps
48"	0.026	7 fps
60" or larger	0.027	7 fps
Corrugated polyethylene smooth interior pipe	0.012	15 fps
Concrete Culverts	0.013	7 fps
Open Channels		
Concrete, trowel finish	0.013	10 fps
Concrete, broom float	0.015	10 fps
Gunite	0.018	10 fps
Riprap, placed	0.030	7 fps
Riprap, dumped	0.035	7 fps
Gabion	0.028	10 fps
New earth (uniform sodded clay)	0.025	7 fps
Existing earth (fairly uniform, some weeds)	0.030	7 fps
Dense growth of weeds	0.040	7 fps
Dense weeds and brush	0.040	7 fps
Swale with grass	0.035	7 fps

## 5.3 Storm Sewers

Plans for all proposed storm sewer construction must be submitted to the Huntington Stormwater Management Board prior to the start of land disturbing activities.

The specifications for the construction of storm sewers shall not be less stringent than those set forth in the latest edition of the *Indiana Department of Transportation Standard Specifications*.

### 5.3.1 Sewer Pipe

Sewer pipe shall be designed and constructed using the following criteria.

#### 5.3.1.1 General Specifications

##### Minimum Size

The minimum size of all storm sewers shall be 12 inches. The rate of release for detention storage shall be controlled by an orifice plate or other devices, subject to approval of the Board, where the 12-inch pipe will not limit the rate of release as required.

##### Grade

Sewer grade shall be such that, in general, a minimum of two feet of cover is maintained over the top of the pipe. Pipe cover less than the two foot minimum may be used with the approval of the Board. This approval must occur prior to land disturbing activities. Uniform slopes shall be maintained between inlets, manholes, and inlets to manholes. Final grade shall be set with the full consideration of the capacity required, sedimentation problems, and other design parameters. Minimum and maximum allowable slopes shall be those capable of producing velocities of two and one-half (2½) and fifteen (15) feet per second, respectively, when the sewer is full flowing.

##### Alignment

Storm sewers shall be straight between manholes in so far as possible. Where long radius curves are necessary to conform to street layout, the minimum radius of curvature shall be no less than 100 feet for sewers 42 inches and larger in diameter. Deflection of pipe sections shall not exceed the maximum deflection recommended by the pipe manufacturer.

The deflection shall be uniform and finished installation shall follow a smooth curve.

#### Flexible Pipe

All flexible storm sewer pipe must meet a deflection of 7.5%.

### **5.3.1.2 Pipe Materials**

#### Concrete Pipe, Plain and Reinforced

Concrete pipes, both plain and reinforced, shall conform in all respects with ASTM C14, AASHO M86, and Federal SS-P-371 for Non-Reinforced Concrete Pipe, ASTM C76 AASHO M170 and Federal S-P-375 for Reinforced Concrete Pipe and ASTM 361 for Reinforced Low Head Concrete Pipe.

#### Corrugated Smooth Walled Interior PVC

Corrugated smooth walled interior PVC shall conform in all respects to the latest edition of ASTM F 794. Ribs shall be annular.

#### Truss Pipe

Truss pipes shall conform in all respects to the latest edition of ASTM D2680 (Non-pressure pipe).

#### Corrugated Smooth Walled Interior High Density Polyethylene Pipe (HDPE)

Corrugated HDPE pipe shall conform to AASHTO M294 Type S for sizes 12" and larger, and AASHTO M252 Type S for sizes smaller than 12". Joints shall be bell and spigot and shall be watertight. Rubber gasket for joints shall conform to ASTM F477.

#### Corrugated Metal Pipe

Corrugated metal pipe shall conform to the requirements of AASHTO Designation M-36 and Indiana Dept. of Transportation Specifications for Aluminized Steel Type.

In addition, the pipe shall be of full circle and shall be fabricated with helical corrugations and a welded seam extending from end to end of each length of pipe. Pipe made first by the lock-seam method with the seam welded later will not be acceptable. The welded seam shall be continuous, utilizing ultra-high frequency resistance equipment. Seams shall be welded in such a manner that they will develop the full strength of the pipe and not affect the shape or nominal diameter of the pipe. Each pipe end shall be fabricated with

two annular corrugations for the purpose of joining pipes together with band couplers.

#### Polyvinyl Chloride (PVC) Pipe

PVC pipe shall be Type PSM conforming to the latest edition of ASTM D3034 for pipe sizes up to 15-inch and ASTM F679 for pipe sizes 18-inches and larger. HOWEVER, no reworked material shall be used and the material shall have a cell classification of 12454-B as defined in the latest edition of ASTM D1782 and shall have an SDR (Standard Dimension Ratio) of not greater than 35.

For depths of bury through fifteen (15) feet a minimum wall thickness of SDR 35 as defined in Section 7.4.1 of ASTM D-3034 is required. For depths of bury greater than fifteen (15) feet, a minimum wall thickness of SDR 26 is required.

### **5.3.1.3 Pipe Installation**

#### Location

Locate pipes according to dimensions on the drawings. Where the sewer location is not located clearly by dimensions on the drawings, locate the sewer where concrete encasement is used, provide not less than 4-inch thickness including that of pipe joints.

#### Laying Pipe

All pipe shall be inspected for soundness and damage that may have occurred during transportation immediately before being lowered into the trench. Any pipe found to be unsound or damaged will be rejected and removed immediately from the work site.

All sound and intact pipe shall be laid accurately to the required line and grade in such a manner as to form a close, concentric joint with the adjoining pipe and to bring the invert of each section to the required grade. Bell holes shall be dug in advance of the pipe being laid as required. The supporting of any pipe on blocks will not be permitted.

Pipe laying shall proceed upgrade, beginning at the lower end of the sewer, unless otherwise approved by the Board.

Each length of section shall be properly pulled or shoved "home" with a winch or come-a-long against the section previously laid to make a proper joint. The pipe shall then be

securely held in position during the backfill operations. Joints shall not be pulled or cramped more than the design of the joint will permit and so as not to injure the conduit.

All open ends of pipes and branches shall be sealed with plugs or bulkheads firmly held in place in a manner acceptable to the City Engineering Department. No special payments will be made for the placement or removal of said plugs or bulkheads. At the end of each days work, the open ends of all pipes shall be satisfactorily protected against the entrance of animals, earth or other materials.

#### Dewatering

Dewatering sufficient to maintain the water level below the surface of the trench bottom shall be accomplished prior to pipe laying and jointing, if not done prior to excavation and placement of the bedding as called for. The dewatering operation, however accomplished, shall be carried out so that it does not destroy or weaken the strength of the soil under or alongside the trench. When the dewatering operation is complete, the trench shall be replaced in such a manner so as not to disturb the pipe and its foundation.

#### Abandoning Pipe or Structures

Where called for on the plans to be abandoned, said sewers or structures shall be permanently plugged or bulkheaded. Where standard "plugs" are available, they shall be employed. For other pipes or structures, the use of brick and mortar or concrete may be used in a manner suitable to the City Engineering Department .

#### Bedding, Rigid Pipe

The following terms apply to all classifications of rigid pipe bedding.

Definition of Terms for Bedding:

Bc = Outside diameter of pipe, in inches

D = Inside diameter of pipe, in inches

d = Depth of bedding material below the pipe bell, in inches

The values of "d", depth of bedding material below the bell of the pipe shall be as follows:

"D" (inside diameter of pipe)	"d" (depth of bedding material) Minimum Requirements
27" and smaller	3"
30" to 60"	4"
66" and larger	6"

Class "A" bedding is that method of bedding in which the conduit is set on "d" inches of concrete in an earthen foundation and encased in concrete up to  $\frac{1}{4}$ " of "Bc" to fit the lower part of the conduit's exterior breadth. The remainder of the conduit is to be surrounded to a height of at least (12) inches above its top by densely compacted granular backfill material carefully placed by hand to completely fill all spaces under and adjacent to the conduit.

The fill is to be tamped thoroughly on each side of the conduit, as far as practicable, shall be in layers not to exceed six (6) inches in thickness.

The concrete used for Class "A" bedding shall be plain concrete with a 28-day compressive strength of 3,000 psi, unless otherwise specified. Refer to "Bedding and Backfill Details" located in **Appendix C**, Stormwater Conveyance Details for further details on Class "A" bedding.

Class "B" bedding is that method of bedding in which the conduit is set on "d" inches of a fine granular material (sand cushion) in an earth foundation, carefully shaped to fit the lower part of the conduit exterior for a width of at least 60% of the conduit's breadth. The remainder of the conduit is to be surrounded to a height of at least twelve (12) inches above its top by densely compacted granular backfill material carefully placed by hand to completely fill all spaces under and adjacent to the conduit. The fill shall be tamped thoroughly on each side and under the conduit, as far as practicable, in layers not to exceed six (6) inches in thickness. Bell excavation is to be provided. Refer to "Bedding and Backfill Details" located in **Appendix C**, Stormwater Conveyance Details for further details on Class "B" bedding.

Class "B" bedding material shall meet the gradation as set forth in the Indiana Dept. of Transportation Standard Specifications, current edition, Section 211, Special Fill and Backfill ("B" Borrow), except that no more than 12% or less than 5% shall pass the No. 200 sieve (silt or clay).

Each pipe shall be laid in Class "B" bedding unless specifically noted otherwise, as shown on the plans and the construction standard drawings.

Class "C" bedding is that method of bedding in which the conduit is set on an earth foundation, carefully shaped to fit the lower part of the conduit exterior for a width of at least 50% of the conduit's breadth. The remainder of the conduit is to be surrounded to a height of twelve (12) inches above its top by lightly compacted granular backfill material carefully around the exterior of the conduit. Bell excavation is to be provided. Refer to "Bedding and Backfill Details" located in **Appendix C**, Stormwater Conveyance Details for further details on Class "C" bedding.

#### Bedding, Flexible Pipe

Each pipe shall be laid in a Class I or Class II bedding, as shown in **Appendix C**, Stormwater Conveyance Details. Pipe bedding installation shall conform to ASTM D 2321.

#### Existing Sewer Removal and Replacement

When applicable, existing sewer lines shall be completely removed and replaced with new sewer lines. The contractor will be required to maintain service during said removal and replacement, which may entail bypass pumping. The contractor shall inform the engineer of the method proposed for maintaining service.

#### Sheet Piling

Permanent or temporary sheet piling shall be provided in the following circumstances: for construction in areas where wide excavations cannot be permitted; for an excavation that is open for an extended period; or where soil conditions dictate to protect adjacent structures, roadways, and utilities.

The section modulus of piling sections shall be as required to function properly as intended.

Piling sections shall be marked for length and sorted and stacked at the job site to prevent distortion and to facilitate proper sequence of setting and driving.

Interlocks shall be protected from becoming obstructed by sand, gravel, mud or other materials.

Pile tips are approved for use at the contractor's discretion.



### 5.3.2 Structures

Structures shall be designed and constructed using the following criteria.

#### 5.3.2.1 General Specifications

##### Manholes

Manholes shall be installed to provide access to continuous underground storm sewers for the purpose of inspection and maintenance. Manholes shall be provided at the following locations:

- Where two or more storm sewers converge;
- At the point of beginning or at the end of a curve, and at the point of reverse curvature (PC, PT, PRC);
- Where pipe size changes;
- Where an abrupt change in alignment occurs;
- Where a change in grade occurs; and
- At suitable intervals in straight sections of sewer.

The maximum distance between storm sewer manholes shall be as follows:

Size of Pipe (inches)	Maximum Distance (feet)
12 to 42	400
48 and larger	600

##### Inlets

Inlets or drainage structures shall be utilized to collect surface water through grated openings and convey the surface water into storm sewers, channels, or culverts. Inlet design and spacing shall be in accordance with Section 7-400 of the *Indiana Department of Transportation Road Design Manual - Volume 1* or other approved design procedures as determined by the City Engineering Department prior to any land disturbing activities. The inlet grate opening must be adequate to accommodate a 10-year flood event with 50% of the sag inlet areas clogged. An overloaded channel from the

sag inlets to the overflow channel, or basin, shall be provided at sag inlets. This will prevent the maximum depth of water ponding in the street sag from exceeding 7 inches.

#### Culverts

Culverts shall be capable of accommodating peak runoff from a 50-year flood event when crossing under a road which is part of the *Indiana Department of Transportation* rural functional classification system and are classified as principal or minor arterial, or minor collector roads.

#### Special Hydraulic Structures

The use of special hydraulic structures shall be limited to those locations justified by prudent planning and by careful and thorough hydraulic engineering analysis. Special hydraulic structures control the flow of water in storm runoff drainage systems. They include: junction chambers, drop manholes, inverted siphons, stilling basins, and other special structures.

### **5.3.2.2 Materials**

#### Reinforced Concrete Manholes

Reinforced concrete manholes shall be erected of precast, or cast in place, reinforced concrete sections to the shape of the manhole. Steps shall be cast in place in accordance with the standards as shown in **Appendix C**, Stormwater Conveyance Details. All concrete, reinforcing and wall thickness shall be in accordance with the latest edition of ASTM Designation C-478. All structure joints shall be watertight and constructed in accordance with the latest edition of ASTM Specification C-443. The bottom of the structures shall be of either precast, poured in place, or monolithic bottom stack, with 3,000 psi concrete to conform to the plans. They shall be at least eight (8) inches thick and reinforced as shown in **Appendix C**, Stormwater Conveyance Details.

#### Precast Manhole Components

Precast manhole components shall conform to the latest edition of ASTM C-478, and to the design dimensions indicated by the approved plans. All precast manhole components shall be manufactured by an experienced and reputable manufacturer whose precast manhole components have been used commercially for at least three (3) years. Cones and sections shall be substantially free from fractures,

large or deep cracks and surface roughness. Slabs shall be sound and free from gravel pockets.

#### Monolithic Concrete Manholes

Monolithic concrete manholes shall conform to the contract drawings and the Stormwater Conveyance Details located in **Appendix C**. Walls and base dimensions shall be of approved thickness and the maximum step spacing shall be sixteen (16) inches.

#### Manhole Joints

Storm sewer manhole joints shall be neatly joined by flexible rubber gasket or approved bitumastic material.

#### Cast Iron Frames and Covers

Cast iron frames and covers shall conform to the requirements of the latest edition of ASTM A48 for Gray Cast Iron. The dimensions, weights and finish preparation shall conform to the appropriate construction standards.

#### Ductile Cast Iron Frames, Covers and Grates

Ductile cast iron frames, covers and grates shall conform to the requirements of the latest edition of ASTM A536 for Ductile Cast Iron. The dimensions, weights and finish preparation shall conform to the appropriate construction standards.

### **5.3.2.3 Installation**

#### Dewatering

Dewatering sufficient to maintain the water level below the surface of the trench bottom shall be accomplished prior to pipe laying and jointing, if not done prior to excavation and placement of the bedding as called for. The dewatering operation, however accomplished, shall be carried out so that it does not destroy or weaken the strength of the soil under or alongside the trench. When the dewatering operation is complete, the trench shall be replaced in such a manner so as not to disturb the pipe and its foundation.

#### Sheet Piling

Permanent or temporary sheet piling shall be provided in the following circumstances: for construction in areas where wide excavations cannot be permitted; for an excavation that is open for an extended period; or where soil conditions dictate to protect adjacent structures, roadways, and utilities.

The section modulus of piling sections shall be as required to function properly as intended.

Piling sections shall be marked for length and sorted and stacked at the job site to prevent distortion and to facilitate proper sequence of setting and driving.

Interlocks shall be protected from becoming obstructed by sand, gravel, mud or other materials.

Pile tips are approved for use at the contractor's discretion.

#### Bedding for Structures

Precast base sections shall be placed on a well-graded granular bedding course conforming to the requirements for sewer bedding, but not less than four (4) inches in thickness and extending to the limits of the excavation. The bedding course shall be firmly tamped and made smooth and level to assure uniform contact and support of the precast element.

#### Cast-in-Place Bases

Unless otherwise specified, cast-in-place bases shall be at least eight (8) inches in thickness and shall extend at least six (6) inches radially outside of the outside dimensions of the manhole section. The cast-in-place base shall be made of 3,000 psi concrete, 28-day compression test, and shall be reinforced as shown on the construction standards.

#### Lift Holes

All lift holes in precast elements shall be thoroughly wetted and be completely filled with non-shrinking concrete grout, smoothed and painted both inside and out, to ensure water tightness.

#### Placing Precast Sections

Precast sections shall be placed and aligned to provide vertical sides and vertical alignment of the ladder rungs. The completed manhole shall be rigid, true to dimensions and watertight.

#### Placing of Castings

Castings placed on a concrete surface shall be set in full grout beds. The grout shall be mixed in proportion of one (1) part Portland Cement to three (3) parts sand, by volume, based on dry materials. Castings shall be set accurately to

the finished elevation so that no subsequent adjustment will be necessary, or unless otherwise specified by the licensed Engineer.

After grout has cured, use an approved bitumastic material around the outside of casting to ensure water tightness.

When working in paved streets or areas which have been brought to grade, not more than fifteen (15) inches shall be provided between the top of the cone or slab and the underside of the manhole casting for adjustment of the casting to street grade.

When working in an unimproved street or alley, not less than twelve (12) inches of adjusting rings shall be provided between the top of the cone or slab and the underside of the manhole casting for adjustment of the casting to finished grade. The top of the manhole casting shall be flush with the finished grade, unless otherwise shown in the plans.

When working in cultivated areas, the top of the manhole casting shall be buried three (3) feet. In non-cultivated areas, the casting shall be flush with the finished grade, unless otherwise directed by the licensed Engineer.

In the event that the last manhole section is a reducing cone set to final grade by the licensed Engineer and if it becomes necessary to lower them below the cone, compensation to the contractor will be allowed for said adjustment and changing of the manhole stacks.

When concrete adjusting rings are used to set the castings to grade, they shall be pointed up and a grout bed placed between each ring and casting; and made watertight with a heavy coating of an approved bitumastic material on the outside of the structure. The casting is flush with the surrounding pavement.

When rubber adjustment rings are used to set castings to grade, they shall be positioned so that the casting is flush with surrounding pavement.

#### Channels and Inverts

Channels and inverts shall be made to conform accurately to the sewer characteristics and grades, and shall be brought together smoothly with well-rounded junctions, satisfactory to

the licensed Engineer and in conformance with the Stormwater Conveyance Details in **Appendix C**.

#### Pipe Connections

Pipes shall be firmly full of jointing material at entrance to manhole to ensure water tightness. The pipes shall not protrude farther than three (3) inches into the inside face of the manhole, measured along the horizontal center of the pipe. Special care shall be taken to see that the opening through which pipes enter the structure have all pipe ends sawed and smoothed completely.

Rubber water stops, "O"-Ring gaskets, or poured-in-place pipe sleeves shall be used for water tightness between the pipe and the manhole for all sidewall pipes.

When new holes are required in the manhole, they shall be core drilled, or star drilled, in a circle of the required diameter and then knocked out. In no instance shall new holes be sledge-hammered out.

#### **5.3.2.4 Grade Adjustment of Existing Structures**

When adjusting castings to grade or reconstructing structures, the applicant shall conform to the applicable provisions of the Indiana Department of Transportation Standard Specifications, current edition.

#### **5.3.3 Joints, Fittings and Appurtenances**

Joints, fittings and appurtenances shall be designed and constructed using the following criteria.

##### Joints

Elastomeric seals for gasketed joints for corrugated and spiral wound PVC shall meet ASTM F477 and ASTM D3212.

Flexible rubber gasket joints for concrete sewer pipe shall conform to the requirements of ASTM Designation C-443, joints for circular concrete sewer and culvert pipe, using flexible watertight, rubber gaskets. Storm sewer pipe larger than 24-inch diameter may be tongue and groove plain joint unless the sewer is under a pavement or the plans specifically say otherwise. If plain joint is used, an approved bitumastic material shall be applied to each joint.

Coupling bands for use with corrugated metal pipe shall be "hugger"

type with “O” rings.

The PVC joint shall conform to ASTM D3212 “push on” type with a confined rubber gasket conforming to ASTM F477.

#### Fittings

PVC sewer fittings shall conform to the requirements of ASTM D-3034 specifications. Four, six, and eight-inch fittings shall be molded in one piece, with elastomeric joints and minimum socket depths as specified in Section 6.2 and 7.3.2 of the D-3034 specification. Fittings 10 inches and larger shall be molded or fabricated from pipe meeting ASTM D-3034 with standard pipe bells and gaskets identified by the manufacturer.

The PVC fittings for corrugated and spiral wound pipe shall conform to the latest edition of ASTM F 794.

#### Plugs

All fittings shall be capped with a plug of the same material as the pipe, and gasketed with the same gasket material as the pipe joint, or be of material approved by the City Engineering Department. The plug shall be able to withstand all test pressures involved without leakage.

### **5.3.4 Construction Requirements**

The following construction requirements must be addressed prior to any land disturbing activities.

#### **5.3.4.1 Surface Conditions**

The applicant shall examine the areas and conditions under which work of this section will be performed. Conditions detrimental to timely and proper completion of the work to be done shall be corrected. Work may not be done until unsatisfactory conditions are corrected.

#### **5.3.4.2 Field Measurements**

The applicant shall make necessary measurements in the field to assure precise fit of items in accordance with the approved design.

### **5.4 Open Channels**

Open channels shall be designed and constructed using the following criteria.

### 5.4.1 Channel Cross-Section and Grade

The required channel cross-section and grade are determined by the design capacity. A minimum depth may be required to provide adequate outlets for subsurface drains, tributary ditches, or streams. The channel grade shall be such that the velocity in the channel is high enough to prevent siltation but low enough to prevent erosion. Velocities less than 1.5 feet per second should be avoided because siltation will take place and ultimately reduce the channel cross section. The maximum permissible velocities in vegetal-lined channels are shown in table 5.4.1. Developments through which the channel is to be constructed must be considered in design of the channel section.

<b>TABLE 5.4.1</b>			
<b>Maximum Permissible Velocities<sup>1</sup> in Vegetal-Lined Channels</b>			
<b>COVER</b>	<b>SLOPE RANGE<sup>2</sup> (percent)</b>	<b>EROSION RESISTANT SOILS (feet per second)</b>	<b>EASILY ERODED SOILS (feet per second)</b>
Bermudagrass	0-5	8	6
	5-10	7	5
	Over 10	6	4
Bahia Buffalograss Kentucky bluegrass Smooth brome Blue grama	0-5	7	5
	5-10	6	4
	Over 10	5	3
Grass mixtures Reed canarygrass	0-5	5	4
	5-10 <sup>2</sup>	4	3
Lespedize sercea Weeping lovegrass Yellow Bluestem Redtop Alfalfa Red fescue	0-5 <sup>3</sup>	3.4	2.5
Common lespedeza Sudangrass <sup>4</sup>	0-5 <sup>4,5</sup>	3.5	2.5
1 – Use velocities exceeding 5 feet per second only where good converse and proper maintenance can be obtained.			
2 – Do not use on slopes steeper than 10 percent except for vegetated side slopes in combination with a stone, concrete, or highly resistant vegetative center section.			
3 – Do not use on slopes steeper than 5 percent except for vegetated side slopes in combination with stone, concrete, or highly resistant vegetative center section.			
4 – Annuals use on mild slopes or as temporary protection until permanent converse are established.			



#### **5.4.2 Side Slopes**

Earthen channel side slopes shall be no steeper than 2 to 1. Flatter slopes may be required to prevent erosion and for ease of maintenance. Where channels will be lined, side slopes shall be no steeper than 1 ½ to 1 with adequate provisions made for weep holes. Side slopes steeper than 1 ½ to 1 may be used for lined channels, provided that the side lining and structural retaining wall are designed and constructed with provisions for live and dead load surcharge.

#### **5.4.3 Peak Flows**

Open channels carrying peak flows greater than thirty (30) cubic feet per second shall be capable of accommodating peak runoff for a 50-year flood event within the drainage easement.

#### **5.4.4 Channel Stability**

The following characteristics are indicative of a stable channel:

- It neither aggrades nor degrades beyond tolerable limits;
- The channel banks do not erode to the extent that the channel cross-section changes appreciably;
- Excessive sediment bars do not develop;
- Excessive erosion does not occur around culverts, bridges, or elsewhere; and
- Gullies do not form or enlarge due to the entry of uncontrolled surface flow to the channel.

Channel stability shall be determined for an aged condition, and the velocity shall be based on the design flow or the bank full flow, whichever is greater, using “n” values for various channel linings. In no case, is it necessary to check channel stability for discharges greater than that from a 100-year flood event.

Channel stability must be checked for conditions immediately after construction. For this stability analysis, the velocity shall be calculated for the expected flow from a 10-year flood event on the watershed, or the bank full flow, whichever is smaller. The “n” value for newly constructed channels in fine-grained soils and sands may be

determined in accordance with the National Engineering Handbook 5, Supplement B, Soil Conservation Service, and shall not exceed channel 0.025.

The allowable velocities for newly constructed channels may be increased by a maximum of 20 percent to reflect the effects of vegetation to be established under the following conditions:

- The soil and site in which the channel is to be constructed are suitable for rapid establishment and support of erosion-controlling vegetation;
- Species of erosion-controlling vegetation adapted to the area and proven methods of establishment are shown; and
- The channel design includes detailed plans for establishment of vegetation on the channel side slopes.

#### **5.4.5 Drainage of Waterways**

Vegetated waterways that are subject to low flows of long duration or where wet conditions prevail shall be drained with a tile system or by other means such as paved gutters. Tile lines may be outlet through a standard tile outlet.

#### **5.4.6 Establishment of New Regulated Drain**

When the City Engineering Department determines it is necessary to establish a new Regulated Drain, each developer must provide the necessary information to meet the requirements of the 1965 Indiana Drainage Code, as amended, for the establishment of a new Regulated Drain. The City Engineering Department shall determine the necessary easements for adequate maintenance of any new Regulated Drain.

#### **5.4.7 Appurtenant Structures**

The design of channels shall provide all structures required for the proper functioning of the channel and the laterals thereto, and travelways for operation and maintenance. Recessed inlets and structures needed for entry of surface and subsurface flow into channels without significant erosion or degradation shall be included in the design of channel improvements. The design is also to provide the necessary flood gates, water level control devices, and any other appurtenances affecting the functioning of the channels and attainment of the purpose for which they are built.

The effect of channel improvements on existing culverts, bridges, buried cables, pipelines, and inlet structures for surface and subsurface drainage on the channel being improved and laterals thereto shall be evaluated to determine the need for modification or replacement. Culverts and bridges which are modified or added as part of channel improvement projects shall meet reasonable standards for the type of structure, and shall have a minimum capacity equal to the design discharge or governmental agency design requirements; whichever is greater.

#### **5.4.8 Disposition of Spoils**

Spoil material resulting from clearing, grubbing, and channel excavation shall be disposed of in such a manner which will:

- Minimize overbank wash;
- Provide for the free flow of water between the channel and flood plain unless the valley routing and water surface profile are based on continuous dikes being installed;
- Not hinder the development of travelways for maintenance;
- Leave the right-of-way in the best condition feasible, consistent with the project purposes for productive use by the owner;
- Improve the aesthetic appearance of the site to the extent feasible; and
- Be approved by the IDNR or the U.S. Army Corps of Engineers (whichever is applicable), if deposited in floodway.

#### **5.4.9 Materials**

Materials acceptable for use as channel lining are:

- Grass;
- Revetment riprap;
- Concrete;
- Hand-laid riprap;
- Pre-cast cement concrete riprap;

- Grouted riprap;
- Gabions;
- Coir logs;
- Mesh matting; or
- Cellular walls.

Other lining materials shall receive specific approval of the City Engineering Department. Materials shall comply with the latest edition of the Indiana Department of Highways' "Standards Specification".

## 6.0 EROSION AND SEDIMENT CONTROL

### 6.1 Introduction

This chapter describes policies, design criteria and information for erosion and sediment control best management practices (BMPs).

#### 6.1.1 Background and Purpose

Land development necessitates the removal of natural ground cover, creating the potential for erosion to occur. Erosion from a construction site negatively impacts water quality, and the ability of stormwater facilities to continue functioning properly.

This chapter is intended to establish minimum standards for the design and construction of erosion and sedimentation control BMPs.

#### 6.1.2 Design Principles

*The Indiana Handbook for Erosion Control in Developing Areas* has identified ten general principles of erosion and sediment control. They are as follows:

- Fit the development to the existing terrain and soil;
- Develop an erosion and sediment control plan before land-disturbing activities begin, and then fully implement the plan during construction;
- Retain existing vegetation on the construction site wherever possible;
- Minimize the extent and duration that bare soil is exposed to erosion by wind and water;
- Retain sediment on-site as much as possible;
- If possible, divert off-site runoff away from disturbed areas;
- Minimize the length and steepness of slopes;
- Stabilize disturbed areas as soon as possible;
- Keep velocity of runoff leaving site low;
- Inspect and maintain erosion control measures regularly.

These general principles shall be used along with specific erosion and sediment control measures during construction activities.

## **6.2 Details on Specific Practices**

In addition to the following practices, the applicant should also consult *The Indiana Stormwater Quality Manual* (formerly *The Indiana Handbook for Erosion Control in Developing Areas*) for detailed design, construction and maintenance criteria for all erosion and sediment control practices

### **6.2.1 Temporary Sediment Basin**

A temporary sediment basin is a temporary impoundment built to retain sediment and debris. Sediment basins prevent off-site sedimentation by retaining sediment on the construction site.

A schematic example of a temporary sediment basin can be found in **Appendix D**, Erosion and Sediment Control Details.

#### **6.2.1.1 Site and Design Considerations**

The following design and site considerations must be followed when designing a temporary sediment basin:

- The basin shall have a depth of at least 3 feet with sufficient surface area to trap the sediment;
- The site shall be of large enough size for the sedimentation basin to have at least one percent of its drainage area with 30 acres being the maximum drainage area;
- The basin shall be able to maintain a three foot minimum depth; and
- The basin shall not create a condition in which the discharge rate from a sedimentation basin would cause scouring in the receiving channel.

#### **6.2.1.2 Materials**

Temporary sediment basin design and material selection is site specific. Materials used must ensure that all design considerations are met.

### **6.2.1.3 Installation**

The following installation procedures must be followed when installing a temporary sediment basin:

- Locate the sediment basin as close to the sediment source as possible, considering soil type, pool area, dam length, and spillway conditions;
- Clear, grub, and strip the dam location, removing all woody vegetation, rocks and other objectionable material;
- Excavate the area (outlet apron first), stockpiling any surface soil having high amounts of organic matter for later use; and
- Clear the sediment pool to facilitate sediment cleanout.

### **6.2.1.4 Maintenance**

The developer is responsible for maintenance and inspections of all temporary sediment basins. The maintenance plan must include, but is not limited to:

- Inspection of the sediment basin after each storm event;
- The removal and proper disposal of sediment that has accumulate to one-half the design volume;
- The removal of trash and other debris from the riser, emergency spillway, and pool area; and
- The removal of the basin after the drainage area has been permanently stabilized, inspected, and approved. This may be done by draining any water, removing the sediment to a designated disposal area, smoothing the site to blend with the surrounding area, and permanent stabilization.

## **6.2.2 Rock Check Dam**

A rock check dam is a small temporary barrier, grade control structure, or dam constructed across a swale, drainage ditch, or area of concentrated flow. The purpose of this structure is to minimize the

erosion rate by reducing the velocity of stormwater and to capture larger soil particles.

A schematic example of a rock check dam can be found in **Appendix D**, Erosion and Sediment Control Details.

#### **6.2.2.1 Site and Design Considerations**

The following design and site considerations must be followed when designing a rock check dam:

- The contributing drainage area for a rock check dam shall not exceed two acres;
- Two or more check dams in series shall be used for drainage areas greater than two acres;
- The maximum spacing between dams shall be such that the toe of the upstream dam is at the same elevation as the top of the downstream dam;
- The dam height shall be 2 feet maximum, measured at the center of the check dam, but at least 9 inches lower in center than the outer edges at natural ground elevation; and
- The side slopes of the check dam shall be 2:1 or less.

#### **6.2.2.2 Materials**

The developer shall use Revetment Riprap, per INDOT Standards, for the construction of a rock check dam.

#### **6.2.2.3 Installation**

The following installation procedures must be followed when installing a rock check dam:

- Excavate a cutoff trench into the ditch banks, and extend it a minimum of 18 inches beyond the abutments;
- Place the rock in the cutoff trench and channel; and



- Extend the rock at least 18 inches beyond the channel banks to keep overflow water from undercutting the dam.

#### **6.2.2.4 Maintenance**

The developer is responsible for the inspection and maintenance of the rock check dam. The maintenance plan must include, but is not limited to:

- The inspection of the dam and channel after each storm event;
- Repairs as a result of damage from a storm event, which are to be performed immediately following the event;
- The installation of a riprap liner in the event that there is significant erosion between dams;
- The removal of sediment accumulated behind each dam as needed to maintain channel capacity, allow drainage through the dam, and to prevent large flows from displacing sediment;
- The addition of rock to the dams, as necessary, to maintain the height and cross section; and
- The removal of rock and stabilized channel, using an erosion resistant lining when necessary, should the dam no longer be needed.

#### **6.2.3 Temporary Stone Construction Entrance**

The purpose of a temporary stone construction entrance is to provide a stable entrance and exit condition from the construction site and to keep mud and sediment off public roads.

A schematic example of a temporary stone construction entrance can be found in **Appendix D**, Erosion and Sediment Control Details.

##### **6.2.3.1 Site and Design Consideration**

The following design and site considerations must be followed when designing a temporary stone construction entrance:

- The temporary entrance must be at least 12-feet wide and 50-feet long or the distance to the foundation;
- If wet conditions are expected, geotextile fabric is required for stabilization; and
- Six inches of clean depth must be maintained.

### **6.2.3.2 Materials**

Materials required for the use of a temporary stone construction entrance include:

- 2 to 3-inch size washed stone (per INDOT standards); and
- Geotextile fabric if wet conditions are expected.

### **6.2.3.3 Installation**

The following installation procedures must be followed when installing a temporary stone construction entrance:

- Avoid locating the entrance on a steep slope or at a curve in a public roads;
- Remove all vegetation and other objectionable material from the foundation area, then grade and crown for positive drainage;
- If slope towards the road exceeds 2%, construct a 6-8 inch high water ridge with 3:1 side slopes across the foundation area about 15 feet from the entrance to divert runoff away from the road; and
- Install a temporary culvert under pad to maintain proper public road drainage, as necessary.

### **6.2.3.4 Maintenance**

The developer is responsible for the inspection and maintenance of the temporary stone construction entrance. The maintenance plan must include, but is not limited to:

- Inspection of the entrance pad and sediment disposal area on a weekly basis, but also following storm events or heavy use;
- Reshaping of the pad for drainage and runoff control, as necessary.
- Topdressing the entrance with clean stone as necessary;
- The immediate removal of mud and sediment tracked or washed onto public roads. This shall be performed by brushing or sweeping; and
- The immediate repair of any broken road pavement.

#### 6.2.4 Silt Fence

A silt fence is a temporary barrier constructed of geotextile fabric, posts, and depending upon the strength of the fabric used, wire fence for support. The purpose of a silt fence is to retain sediment on-site by reducing the velocity of sheet flow.

A schematic example of a silt fence can be found in **Appendix D**, Erosion and Sediment Control Details.

##### 6.2.4.1 Site and Design Considerations

The following design and site considerations must be followed when designing a silt fence:

- The contributing drainage area for a silt fence shall be limited to  $\frac{1}{4}$  acre per 100 ft of fence and further limited by slope steepness as shown in the following table;

Land Slope	Max Distance Above Fence
< 2%	100 ft
2 – 5%	75 ft
5 – 10%	50 ft
10 – 20%	25 ft
> 20%	15 ft

- The fabric must be buried at an 8 inch minimum depth;

- The fence shall be placed on the contour to avoid channelization; and
- The spacing of the posts shall be 8 foot maximum if supported by wire and 6 foot maximum if no supporting wire is used.

#### **6.2.4.2 Materials**

Materials required for the installation of a silt fence include:

- 2-inch x 2-inch hardwood posts or steel fence posts;
- Woven or non-woven geotextile fabric;
- When applicable, a 14 gauge, 6-inch mesh wire fence.

#### **6.2.4.3 Installation**

The following installation procedures must be followed when installing a silt fence:

- Install fence parallel to the contour of the land;
- Dig an 8-inch deep flat bottomed or V-trench along the entire fence line;
- Extend ends upslope into the trench to allow water to pond behind fence;
- Install fence with posts on the down slope side;
- Join silt fence sections using a wrap joint; and
- Backfill the trench with compacted earth or gravel.

#### **6.2.4.4 Maintenance**

The developer is responsible for the inspection and maintenance of all silt fences. The maintenance plan must include, but is not limited to:

- The frequency of inspections;
- The requirement for repairs and replacements as necessary;

- The removal of sediment deposits when they reach one half the fence height or in the event that they cause the fabric to bulge; and
- The removal and proper disposal of all fences. Once removed, the depressions shall be filled, compacted, and seeded.

### 6.2.5 Straw Dam

A straw dam is a temporary sediment barrier constructed of straw bales across small drainages. The purpose of a straw dam is to retain sediment on-site by reducing the velocity of sheet flow.

A schematic example of a straw dam can be found in **Appendix D**, Erosion and Sediment Control Details.

#### 6.2.5.1 Site and Design Considerations

The following design and site considerations must be followed when designing a straw dam:

- The contributing drainage area for a straw dam shall be limited to ¼ acre per 100 ft of dam and further limited by slope steepness as shown in the following table.

Land Slope	Maximum Spacing
Less than 2%	100 ft
2 – 5%	75 ft
5 – 10%	50 ft
10 – 20%	25 ft
Greater than 20%	15 ft

- The dam shall be located on the contour to prevent channelization; and
- The straw dam shall be anchored to prevent movement.

#### 6.2.5.2 Materials

Materials required for the installation of a straw dam include:

- New, firm, and well compacted straw bales bound with wire or nylon. Minimum size for bales shall be 14" x 18" x 36"; and
- 2-inch x 2-inch inch hardwood stakes.

### **6.2.5.3 Installation**

The following installation procedures must be followed when installing a straw dam:

- Excavate trench at least 4 inches deep, a bale's width, and long enough that the end bales are somewhat upslope of the sediment pool;
- Place each bale in the trench so the bindings are oriented around the sides rather than top and bottom;
- Anchor the dam by driving stakes through each bale until nearly flush with the top;
- Insert straw into any gaps between bales to prevent sediment-laden water from running through; and
- Backfill and compact the excavated soil against the bales to the ground level on the down-slope side and to 4 inches above ground level on the up-slope side.

### **6.2.5.4 Maintenance**

The developer is responsible for the inspection and maintenance of all straw dams. The maintenance plan must include, but is not limited to:

- The frequency of inspections of the dams;
- The requirement for repairs and replacements when necessary;
- The removal of sediment deposits not to undermine the entrenched bales.
- The removal and proper disposal of all dams. Once removed, the depressions shall be filled, compacted, and seeded.

## **6.2.6 Fabric Drop Inlet Protection**

Fabric drop inlet protection is a temporary woven geotextile barrier placed around a drop inlet to prevent sediment from entering the storm drains during construction operations.

A schematic example of fabric drop inlet protection can be found in **Appendix D**, Erosion and Sediment Control Details.

### **6.2.6.1 Site and Design Considerations**

The following design and site considerations must be followed when designing fabric drop inlet protection:

- The height of the fabric shall be a maximum of 1.5 feet and a minimum of 1 foot;
- The base of the fabric shall be buried at least 6 inches below the ground surface;
- The support posts shall have a minimum length of 3 feet;
- The maximum spacing of the support posts shall be 3 feet;
- Framing shall be used to connect tops of posts to stabilize the structure; and
- The maximum contributing drainage area for fabric drop inlet protection is 1 acre.

### **6.2.6.2 Materials**

Materials required for the installation of fabric drop inlet protection include:

- Steel fence posts or 2-inch x 4-inch wooden posts; and
- Durable, high-strength synthetic woven fabric.

### **6.2.6.3 Installation**

The following installation procedures must be followed when installing fabric drop inlet protection:

- Set the top of the fabric at least 6 inches below the downslope ground elevation;
- Cut the fabric from a single roll to eliminate joints if possible. If joints are needed, a wrap joint shall be used;
- Bury the bottom of the fabric at least 1 foot deep, backfill, and compact the backfill; and
- Space the support posts evenly against the inlet perimeter a maximum of 3 feet apart and drive them about 1.5 feet into the ground.

#### **6.2.6.4 Maintenance**

The developer is responsible for the inspection and maintenance of all fabric drop inlet protection sites. The maintenance plan must include, but is not limited to:

- Inspection of the fabric barrier after storm events and making the needed repairs at that time;
- The removal of sediment from the pool area to provide storage for the next storm, while avoiding damage or undercutting of the fabric during sediment removal.
- The removal and proper disposal of all construction material and sediment. The area shall then be graded to the elevation of the top of the inlet, and stabilized.

### **6.2.7 Sandbag Curb Inlet Protection**

The purpose of sandbag curb inlet protection is to trap sediment on paved streets that receive relatively small runoff flows, preventing the sediment from being transported further down the street or into an inlet.

A schematic example of a sandbag curb inlet protection can be found in **Appendix D**, Erosion and Sediment Control Details.

#### **6.2.7.1 Site and Design Considerations**

The following design and site considerations must be followed when designing a sandbag curb inlet protection:



- 1 to 3 layers of sandbags shall be used;
- The length of the inlet protection structure shall be a minimum of 3 feet; and
- The maximum contributing drainage area is 1 acre.

#### **6.2.7.2 Materials**

All sandbags used for curb inlet protection shall be filled half full with sand or fine gravel.

#### **6.2.7.3 Installation**

The following installation procedures must be followed when installing sandbag curb inlet protection:

- Lay the bags tightly end to end in a row curving from the curb and away from the inlet, upslope from the curb inlet;
- Overlap the barrier onto the curb, and extend it into the street to intercept the runoff;
- If using more than one row, overlap the bags with the row beneath, and leave a one-bag gap in the middle of the top row to serve as a spillway.

#### **6.2.7.4 Maintenance**

The developer is responsible for the inspection and maintenance of all sandbag curb inlet protection sites. The maintenance plan must include, but is not limited to:

- The frequency of inspection for damage by vehicular traffic and following each storm event;
- The repair and replacement procedures when necessary; and
- The removal of sediment when it reaches half the height of the barrier.

## **6.2.8 Riprap Chute Outfall Protection**

Riprap chute outfall protection is a pad or apron of rock placed at the outlet end of culverts or chutes. They are used to reduce velocity and prevent erosion at the outlet of a channel or culvert.

A schematic example of riprap chute outfall protection can be found in **Appendix D**, Erosion and Sediment Control Details.

### **6.2.8.1 Site and Design Considerations**

The following design and site considerations must be followed when designing riprap chute outfall protection:

- The maximum contributing drainage area shall be 100 acres;
- The minimum thickness shall be 12 inches with a geotextile foundation and
- Ensure that the apron size is proportional to the pipe diameter size.

### **6.2.8.2 Materials**

Riprap chute outfall protection materials shall be hard, angular, and highly weather resistant riprap stone of size and gradation that will withstand the velocities of the chute.

### **6.2.8.3 Installation**

The following installation procedures must be followed when installing riprap chute outfall protection:

- Excavate the apron area subgrade below design elevation to allow for thickness of the filter and riprap;
- Place fabric on compacted and smooth foundation and install the riprap;
- Make sure the top of the riprap apron is level with or slightly below the receiving stream;

- Blend the riprap smoothly to the surrounding grade; and
- Stabilize all disturbed areas immediately following installation.

#### **6.2.8.4 Maintenance**

The developer is responsible for the inspection and maintenance of all riprap chute outfall protection sites. The maintenance plan must include, but is not limited to:

- The inspection of rock chutes after storm events for stone displacement and for erosion at the sides and ends of the apron; and
- Making needed repairs immediately using the appropriately sized stone, while placing them at or below the finished grade.

## **7.0 POST-CONSTRUCTION STORMWATER QUALITY**

### **7.1 Introduction**

This chapter describes policies, design criteria and information for water quality best management practices (BMPs) as required by the City of Huntington's National Pollutant Discharge Elimination System (NPDES) permit.

#### **7.1.1 Background and Purpose**

Best management practices (BMPs), both structural and non-structural, can reduce the amount of pollutants in stormwater. This section of the manual establishes minimum standards for the design, maintenance, application, and construction of water quality BMPs. The information provided in this chapter establishes performance criteria for stormwater quality management and procedures to be followed when preparing a BMP plan that must be in compliance with this manual.

BMPs noted in this chapter refer to post-construction BMPs, to be used after construction has been completed and the site has been stabilized. The installation of certain BMPs, such as bioretention areas and sand filters, prior to stabilization can cause failure of the measure due to clogging from sediment caused during land disturbing activities. Nonetheless, with a strict construction sequence, detention ponds and other BMPs can be installed initially during construction and used as sediment control measures. In those instances, the construction sequence must require that the pond is cleaned out with pertinent elevations and storage and treatment capacities re-established as noted in the approved post-construction stormwater management plan.

#### **7.1.2 Stormwater Quality Control Requirements**

The City of Huntington has adopted a policy that city-wide control of stormwater runoff quality will be based on the management of total suspended solids (TSS). The target TSS removal rate is 80%.

In addition to TSS removal, BMPs must also be designed to treat the water quality volume (WQv) or the first flush of runoff.

#### **7.1.3 Water Quality Volume**

The storage needed to capture and treat the runoff from the first one inch of rainfall is referred to as Water Quality Volume (WQv). In numerical terms, it is equivalent to an inch of rainfall multiplied by the volumetric runoff coefficient (Rv) and the site area.

The following equation is used to calculate WQv (in acre-feet):

$$WQv = \frac{(P) (Rv) (A)}{12}$$

Where:

WQv = water quality volume (acre-feet)

P = 1 inch of rainfall

Rv = 0.05 + 0.009(I) where I is the percent impervious cover

A = area in acres

All new development projects requiring stormwater quality management (Category 1 Projects) shall be required to treat the first one inch of rainfall. Redevelopment projects will be required to obtain the same approval if the redevelopment is to disturb more than 1 acre.

#### **7.1.4 Structural Best Management Practices**

Table 7.1.4-A, at the end of section 7.1, identifies pre-approved structural BMPs that can be used in Huntington for water quality control when designed and constructed in the manner in which they are intended. Note that many of these measures can also be designed to meet the water quantity control requirements. Specific water quality design requirements are presented in the following sections.

Table 7.1.4-B, at the end of section 7.1, discusses BMP selection criteria based on current and planned use of the site. Note that other approved BMPs and combinations of approved BMPs may be used on a specific site if the target TSS removal rate of 80% is met.

##### **7.1.4.1 Innovative or Otherwise Unapproved BMPs**

All plans for unapproved BMPs (BMPs not included in Table 7.1.4-A) must be certified by a professional engineer, land surveyor, or landscape architect and submitted to the City Engineering Department for approval prior to installation. ASTM standard methods must be followed when verifying performance of new measures. New BMPs must meet the 80% TSS removal rate and must have a low to medium maintenance requirement to be considered by the City.

##### **7.1.4.2 Operation and Maintenance of Structural BMPs**

Each BMP must have an operations and maintenance plan. The maintenance plan must be submitted with the post-

construction stormwater management plan and approved by the City. The approved operations and maintenance plan will be returned to the BMP owner, who will then be required to implement the operations and maintenance plan.

The City must be notified of any changes in BMP ownership, major repairs or BMP failure in writing within 30 days.

In the event that the City finds a BMP in need of maintenance or repair, the City will notify the BMP owner of the necessary maintenance, or repairs, and give the landowner a timeframe for complying with their request. If the maintenance, or repairs, are not completed within the designated timeframe, the City shall perform the repairs or maintenance and bill the landowner for the actual costs for the work.

**Table 7.1.4-A Pre-Approved BMPs**

BMP Type	Description	Quantity Control	WQv and 80% TSS removal
<p><i>Ponds</i></p> <ul style="list-style-type: none"> <li>• Wet Pond</li> <li>• Wet extended detention pond</li> <li>• Micropool extended detention pond</li> <li>• Multiple pond systems</li> </ul>	<p>Stormwater ponds are constructed stormwater retention basins with a permanent pool (or micropool) of water. Runoff from each rain event is captured and treated in the pool.</p>	<p>Yes</p>	<p>Yes</p>
<p><i>Detention Basins</i></p> <ul style="list-style-type: none"> <li>• Detention Basin</li> </ul>	<p>A dry detention basin is an area used to detain stormwater for a relatively short period of time. The area should be dry between storms. The basin allows particles and pollutants to settle.</p>	<p>Yes</p>	<p>No</p>
<p><i>Stormwater Wetlands</i></p> <ul style="list-style-type: none"> <li>• Shallow wetland</li> <li>• Extended detention wetland</li> <li>• Pond/wetland systems</li> <li>• Pocket wetland</li> </ul>	<p>Stormwater wetlands are constructed, artificial wetland systems used for stormwater management. They consist of a combination of shallow marsh areas, open water and semi-wet areas above the permanent pool.</p>	<p>Yes</p>	<p>Yes</p>
<p><i>Bioretention Areas</i></p>	<p>Bioretention areas are shallow stormwater basins or landscaped areas that utilize engineered soils and vegetation to capture and treat stormwater run-off.</p>	<p>No</p>	<p>Yes</p>
<p><i>Sand Filters</i></p> <ul style="list-style-type: none"> <li>• Surface sand filter</li> <li>• Perimeter sand filter</li> </ul>	<p>Sand filters are multi-chamber structures designed to treat stormwater run-off through filtration, using a sand bed as its primary filter media.</p>	<p>No</p>	<p>Yes</p>
<p><i>Water Quality Swales</i></p> <ul style="list-style-type: none"> <li>• Dry Swale</li> </ul>	<p>Water quality swales are vegetated open channels that are designed and constructed to capture and treat stormwater run-off within dry cells.</p>	<p>No</p>	<p>Yes</p>
<p><i>Biofilters</i></p> <ul style="list-style-type: none"> <li>• Filter strip</li> <li>• Grass channel</li> </ul>	<p>While biofilters provide some filtering of stormwater runoff, by themselves they cannot meet the 80% TSS removal performance goal. <i>These measures can only be used as pre-treatment measures or as part of a treatment train.</i></p>	<p>No</p>	<p>No</p>
<p><i>Catch Basin Inserts</i></p> <ul style="list-style-type: none"> <li>• Various designs</li> </ul>	<p>Catch basin inserts are small filtering devices installed in each catch basin to trap suspended solids and other pollutants.</p>	<p>No</p>	<p>Yes</p>

**Table 7.1.4-B BMP Selection Criteria**

Current Use	Planned Use	Approved BMPs
Open land	Commercial strip, light industrial, institutional (individual lots)	Bioretention, wet pond, detention basin, artificial wetland, sand filters, biofilter, water quality swale, catch basin insert
Open land	Commercial or industrial subdivision (regional stormwater plan)	Wet pond, detention basin, wetland
Open land	Residential	Bioretention, wet pond, detention basin, artificial wetland, biofilter, water quality swale
Commercial building or strip (medium imperviousness)	Commercial building or strip	Bioretention, sand filter, catch basin insert, wet pond, detention basin, wetland
Commercial building or strip (small lot, high imperviousness)	Commercial building or strip	Bioretention, sand filter, catch basin insert
Transportation infrastructure	Increased/expanded transportation infrastructure	Water quality swales, wet ponds, detention, basin, artificial wetlands, catch basin inserts

## 7.2 Pre-Approved BMPs

In addition to the following practices, the applicant should also consult *The Indiana Stormwater Quality Manual* (formerly *The Indiana Handbook for Erosion Control in Developing Areas*) for detailed design, construction and maintenance criteria for all post-construction stormwater management practices.

### 7.2.1 Stormwater Ponds

Wet stormwater ponds can be designed to meet both water quality and water quantity requirements. If the retention pond is to be designed for only water quality purposes, then the pond shall be designed to capture the water quality volumes as noted in Section 7.1.4.

A schematic example of stormwater ponds and variations can be found in **Appendix E**, Post-Construction Stormwater Quality Details.



### 7.2.1.1 Site and Design Considerations

The following design and site considerations must be followed when designing a stormwater pond:

- Design the pond with a minimum length to width ratio of 3:1, preferably expanding outward toward the outlet;
- Basins containing permanent ponds shall have a water area of at least one-half acre. If fish are to be maintained in the pond, a minimum depth of approximately 10 feet shall be maintained over at least 25 percent of the pond area. The remaining pond area shall have no extensive shallow area;
- In excavated ponds, the underwater side slopes in the pond shall be stable. In the case of valley storage, natural slopes may be considered to be stable;
- A safety ledge of 4 to 6 feet in width is required and must be installed in all ponds approximately 30 to 36 inches below the permanent water level. In addition, a similar maintenance ledge 12 to 18 inches above the permanent water line shall be provided. The slope between the two ledges shall be stable and of a material such as stone or riprap which will prevent erosion due to wave action;
- A safety ramp exit from the pond is required in all cases and shall have a minimum width of 20 feet and exit slope to 6 horizontal to 1 vertical. The ramp shall be of material that will prevent its deterioration due to vehicle use and/or wave action;
- Maximize flow length between the inlet and outlet structure. Use baffles if short-circuiting cannot be prevented with inlet-outlet placement. Long flow paths and irregular shapes are recommended;
- In areas where there is a contributing drainage area, one must design the BMP as if the entire upstream watershed is fully developed;
- In areas where flow from the upstream watershed bypasses the proposed development, the design of the

BMP will only need to consider the drainage from the site;

- Provide a sediment forebay, or similar pretreatment, upstream from the BMP inlet;
  - The forebay must be sized to contain 0.1 inches of runoff per impervious acre of contributing drainage. The forebay storage volume counts toward the total water quality storage requirements.
  - Exit velocities from the forebay must be non-erosive.
  - Direct maintenance access for appropriate equipment must be provided to the forebay.
  - The bottom of the forebay may be hardened (e.g. using concrete, paver blocks, etc.) to make sediment removal easier.
  - A fixed vertical sediment depth marker must be installed in the forebay to measure sediment deposition over time.
  - Sediment removal in forebay shall occur when 50% of the total capacity has been lost.
  
- All excavated excess spoil may be spread so as to provide for aesthetic and recreational features such as sliding hills, sports fields, etc. Side slopes shall be no greater than 2:1 for safety, erosion control, stability and ease of maintenance;
  
- Rip-rap protection must be provided (or other suitable erosion control means) for the outlet and all inlet structures into the pond;
  
- The minimum drainage area, contributing or effective, for stormwater ponds is 25 acres. The minimum drainage area, contributing or effective, for a micro-pool extended detention facility is 10 acres;
  
- Anti-seep collars or filter diaphragms must be provided for the barrel of principal spillway;
  
- If reinforced concrete pipe is used for the principal spillway, "O"-ring gaskets (ASTM C361) shall be used to create watertight joints;
  
- Provide a one (1) foot minimum freeboard above the maximum anticipated flow depth through the emergency spillway;

- Design and install an emergency drain (i.e. sluice gate or drawdown pipe) capable of draining within 24 hours;
- Design an emergency spillway to pass 1.25 times the peak discharge and peak flow velocity from the 100-year storm event for the entire contributing drainage area (unless the site has been bypassed), assuming post-development conditions. The overflow facility shall be of such a design that its operation is automatic and does not require manual attention;
- Provide trash racks, filters, hoods or other debris control;
- Provide a permanent benchmark within the permanent pool and sediment forebay for sediment removal;
- The principal spillway/riser system must incorporate anti-floatation, antivortex, and trash-rack designs;
- To prevent drawdown of the permanent pool, an impervious soil boundary may be needed;
- Orifice-type outlets are not allowed below the permanent pool elevation of wet ponds and micropools;
- Outlet control structures shall be designed to operate as simply as possible and shall require little or no maintenance and/or attention for proper operation. They shall limit discharges into existing or planned down stream channels or conduits so as not to exceed the predetermined maximum authorized peak flow rate;
- All stormwater detention and retention facilities shall be separated by not less than 25 feet from any building or structure to be occupied, and 10 feet from any right-of-way line, sanitary sewer, or potable water line;
- Safety screens having a maximum opening of 4 inches shall be provided for any pipe or opening to prevent children or large animals from crawling into the structures;
- Danger signs shall be mounted at appropriate locations to warn of deep water, possible flooding during storm

periods and other dangers that exist. Fencing shall be provided.

- Construction debris cannot be disposed of within the facility or used as fill in the embankment; and
- If the pond is used as a sediment control measure during active construction, the sediment must be cleaned out of the pond and elevations and grades reestablished as noted in the approved post-construction stormwater management plan.

### 7.2.1.2 Performance Standards

Wet ponds and variations designed, constructed and maintained as noted above shall provide the following pollutant reductions:

Pollutant	Percent Reduction
BOD	30%
TSS	85%
Total P	50%
Total N	30%
Metals	30%

### 7.2.1.3 Variations

**Wet extended detention ponds:** A wet extended detention pond is a wet pond where the water quality volume is split evenly between the permanent pool and the extended detention storage provided above the permanent pool. During storm events, water is detained above the permanent pool and released over 12 - 48 hours. This design has similar pollutant removal to a traditional wet pond, but consumes less space.

**Micropool extended detention pond:** The micropool extended detention pond is a variation of the wet extended detention pond where only a small micropool is maintained at the outlet to the pond. The outlet is sized to detain the water quality volume for 24 hours. The micropool prevents resuspension of previously settled sediments.

**Multiple pond systems:** Multiple pond systems consist of constructed facilities that provide water quality and quantity volume storage in two or more cells. The additional cells can create longer pollutant removal pathways and improved downstream protection.

#### **7.2.1.4 Advantages**

The advantages of stormwater ponds are as follows:

- High pollutant removal;
- High community acceptance, if designed and maintained correctly;
- Opportunity for wildlife habitat; and
- Multi-objective use for water quality and quantity control.

#### **7.2.1.5 Disadvantages**

The disadvantages of stormwater ponds are as follows:

- Potential for thermal impacts downstream; and
- Dam height restrictions.

#### **7.2.1.6 Maintenance**

The developer is responsible for the inspection and maintenance of each stormwater pond. Each BMP must have an operations and maintenance plan. The BMP owner must maintain and update the BMP operations and maintenance plan, as needed. The operations and maintenance plan must include, but is not limited to:

- The removal of debris from inlet and outlet structures;
- The removal of invasive vegetation from all side slopes;
- The removal of sediment accumulation from forebay and permanent pool area when it is 50% full; and
- The removal of woody vegetation from the embankment.

### **7.2.2 Detention Basin**

A dry detention basin is an area used to detain stormwater for a relatively short period of time. The area shall be dry between storms.

The basin allows particles and pollutants to settle. However, the TSS removal provided is less than 80%. Therefore, dry detention basins must be used in a treatment train in order to provide the 80% TSS removal performance goal.

A schematic example of dry detention basins can be found in **Appendix E**, Post-Construction Stormwater Run Off Control Details.

#### **7.2.2.1 Site and Design Considerations**

The following design and site considerations must be followed when designing a detention basin:

- The seasonal high water table and bedrock shall be at least four feet beneath the bottom of the system to minimize the potential for ground water contamination and to assure that the bottom is dry;
- The maximum volume of water stored and subsequently released at the design release rate shall not result in storage duration in excess of 48 hours unless additional storms occur within that period;
- Use a sediment forebay at all inflow points to trap sediments and allow for easy removal;
- The length of the basin shall be at least three times the width, with the basin narrow at the inlet and wide at the outlet;
- All excavated excess spoil may be spread so as to provide for aesthetic and recreational features such as sliding hills, sports fields, etc. Side slopes shall be at no steeper than 2:1 for safety, erosion control, stability and ease of maintenance;
- Safety screens having a maximum opening of 4 inches shall be provided for any pipe or opening to prevent children or large animals from crawling into the structure;
- Danger signs shall be mounted at appropriate locations to warn of deep water, possible flooding conditions during storm periods and other dangers that exist. Fencing shall be provided.

- The basin floor shall be flat with a 2% slope toward the outlet;
- The maximum planned depth of stormwater stored shall not exceed four feet;
- All stormwater detention facilities shall be separated by not less than 25 feet from any building or structure to be occupied, and 10 feet from any right of way line, sanitary sewer, or potable water line;
- Provisions shall be incorporated to facilitate complete interior drainage of dry bottom basins, to include the provisions of natural grades to outlet structures, longitudinal and transverse grades to perimeter drainage facilities, paved gutters, or the installation of subsurface drains;
- Outlet control structures shall be designed to operate as simply as possible and shall require little or no maintenance and/or attention for proper operation. They shall limit discharges into existing or planned down stream channels or conduits so as not to exceed the predetermined maximum authorized peak flow rate;
- The basin shall, whenever possible, be designed to serve a secondary purpose or multipurpose function. Recreational facilities, aesthetic qualities (open spaces) or other types of use shall be considered in planning the detention facility;
- Grass or other suitable vegetative cover shall be provided throughout the entire basin area; and
- If the basin is used as a sediment control measure during active construction, the sediment must be cleaned out of the basin and elevations and grades reestablished as noted in the approved post-construction stormwater management plan.

### **7.2.2.2 Performance Standards**

Dry detention basins designed, constructed and maintained as noted above shall provide the following pollutant reductions:

<b>Pollutant</b>	<b>Percent Reduction</b>
TSS	60%
Total P	20%
Total N	20%
Metals	40%

### **7.2.2.3 Advantages**

The primary advantage of detention basins is that they may be used as secondary uses (e.g. Parks) because they are dry between storms.

### **7.2.2.4 Disadvantages**

The disadvantages of detention basins are as follows:

- Poor stormwater treatment effectiveness;
- They have the potential to serve as mosquito breeding ground if they are not maintained properly; and
- They require a relatively high level of maintenance.

### **7.2.2.5 Maintenance**

The developer is responsible for the inspection and maintenance of each detention basin. Each BMP must have an operations and maintenance plan. The BMP owner must maintain and update the BMP operations and maintenance plan, as necessary. The operations and maintenance plan must include, but is not limited to:

- The frequency of the removal of accumulated solids, debris, and litter from the detention area. As a general note, sediment shall be removed when it is dry in which case they will crack and are then able to be separated from the bottom and surrounding vegetation;
- The removal of debris from the control device, especially if it has a small orifice;
- The mowing and removal of excess vegetation; and
- The vegetative stabilization of eroding sides or bottom.



### 7.2.3 Stormwater Wetlands

Stormwater wetlands are artificial wetlands created for the purposes of stormwater pollutant removal and stormwater quantity control. It is the intent of the City to encourage regional stormwater wetlands and discourage artificial wetlands designed for individual sites. However, BMP plans proposing stormwater wetlands will be reviewed on a case-by-case basis to determine their feasibility.

A schematic example of stormwater wetlands and variations can be found in **Appendix E**, Post-Construction Stormwater Quality Details.

#### 7.2.3.1 Site and Design Considerations

Prior to the approval of a developments post-construction stormwater management plan, the following design and site considerations must be addressed:

- A water balance must be performed to demonstrate that a stormwater wetland could withstand a thirty-day drought at summer evaporation rates without completely drawing down. Also, inflow of water must be greater than that leaving the basin by infiltration or exfiltration. The following water balance equation shall be used in calculations:

$$S = Q_i + R + Inf - Q_o - ET$$

Where:

S = net change in storage

$Q_i$  = stormwater runoff inflow

R = contribution from rainfall

Inf = net infiltration (infiltration - exfiltration)

$Q_o$  = surface outflow

ET = evapotranspiration

- The wetland must be designed for an extended detention time of 48 hours for the water quality volume (WQv). WQv is addressed in Section 7.1.4. The orifices used for extended detention will be vulnerable to blockage from plant material or other debris that will enter the basin with stormwater runoff. Therefore, some form of protection against blockage (e.g. non-corrodible wire mesh) must be installed;

- The frequently flooded zone surrounding the wetland must be located within the permanent easement;
- The surface area of the wetland must account for a minimum of 1 percent of the area of the watershed draining into it. A minimum of 1.5 percent shall be required for a shallow marsh design. Irregardless, the length to width ration must be at least 2:1;
- The design must incorporate long flow paths through the wetland, as appropriate;
- A forebay shall be established at the pond inflow points to capture larger sediments and be 4 to 6 feet deep. The depth of the forebay shall contain approximately 10 percent of the total volume of the normal pool. Direct maintenance access to the forebay must be provided with an access with a 25 foot minimum width and a maximum slope of 5:1. Permanent sediment depth markers must be provided;
- If high water velocity is a potential problem, some type of energy dissipation device must be installed;
- Site preparation: Soil types conducive to wetland vegetation shall be used during construction. The wetland must be designed to allow slow percolation of the runoff through the substrate. A layer of clay must be added for porous substrates. Ensure that the substrate, once flooded, is soft enough to permit relatively easy insertion of the plants;
- Surrounding slopes must be stabilized with vegetation to aid in trapping pollutants and preventing them from entering the wetland;
- Maintain the wetland to prevent loss of area of ponded water available for emergent vegetation due to sedimentation and/or accumulation of plant material;
- Obtain local assistance for specifications on plants to be used, planting schedule, soil requirements, mulch requirements, etc;
- Construction debris cannot be disposed of in the facility or used as fill in the embankment;

- If the wetland area or sediment forebay is used as a sediment control measure during active construction, the sediment must be cleaned out of the wetland or forebay and elevations and grades re-established as noted in the approved post-construction stormwater management plan;
- Stormwater wetlands must be designed with the recommended proportion of depths noted in Table 7.2.3-A.

The four basic depths and descriptions are:

- *Deepwater*: 1.5 - 6 feet below normal pool elevation. Includes the outlet micropool and deep water channels through the wetland. This zone supports little emergent wetland vegetation but may support floating or submerged vegetation.
- *Low marsh*: 6-18 inches below normal pool elevation or water surface elevation. This zone is suitable for the growth of several emergent wetland species.
- *High marsh*: 6 inches or less below normal pool elevation. This zone will support a greater density and diversity of wetland vegetation than the low marsh. The high marsh area shall have a greater surface area to volume ration than the low marsh area.
- *Semi-wet zone*: Areas above normal pool elevation inundated by larger storm events. This area supports vegetation that can survive periodic flooding.

**Table 7.2.3-A Minimum Required Design Configuration for Stormwater Wetlands**

<b>Design Criteria</b>	<b>Shallow Wetland</b>	<b>Pond/Wetland</b>	<b>Pocket Wetland</b>
Length to width ratio (min)	2:1	2:1	2:1
Allocation of WQv (pool/marsh) in %	25/75	70/30 (includes pond volume)	25/70
Allocation of surface area (deepwater/low marsh/high marsh/semi-wet) in %	20/35/40/5	45/25/25/5 (includes pond surface area)	10/45/40/5
Forebay	Required	Required	Optional
Micropool	Required	Required	Required
Outlet configuration	Reverse-slope pipe or hooded broad crest weir	Reverse-slope pipe or hooded broad crest weir	Hooded broad crest weir

Modified from Massachusetts DEP, 1997; Schueler, 1992

### 7.2.3.2 Performance Standards

Artificial wetlands designed, constructed and maintained as noted above shall provide the following pollutant reductions:

<b>Pollutant</b>	<b>Percent Reduction</b>
BOD	55%
TSS	95%
Total P	55%
Total N	45%
Metals	80%

### 7.2.3.3 Maintenance

The developer is responsible for the inspection and maintenance of each artificial wetland. Each BMP must have an operations and maintenance plan. The BMP owner must maintain and update the BMP operations and maintenance plan, as necessary. The operations and maintenance plan must include, but is not limited to:

- The maintenance of the wetland to prevent loss of area of ponded water available for emergent vegetation due to sedimentation and accumulation of plant material;

- The cleaning of sediment forebays when they are 50% full. Pocket wetlands without forebays must be cleaned after a six-inch accumulation of sediment;
- The maintenance of the ponded water area by raising the elevation of the water level in the permanent pond, by raising the height of the orifice in the outlet structure, or by removing accumulated solids by excavation;
- Water levels may need to be supplemented or drained periodically until vegetation is fully established; and
- It may be desirable to remove contaminated sediment bottoms or to harvest above ground biomass and remove it from the site to permanently remove pollutants from the wetland.

#### 7.2.4 Bioretention

Bioretention areas, or rain gardens, are structural stormwater controls that capture and temporarily store the WQv using soils and vegetation in landscaped areas to remove pollutants from stormwater runoff. Bioretention areas are engineered facilities in which runoff is conveyed as sheet flow to the "treatment area," consisting of a grass buffer strip, ponding area, organic or mulch layer, planting soil, and vegetation. An optional sand bed can be included in the design to provide aeration and drainage of the planting soil. The filtered runoff is typically collected and returned to the conveyance system, though it can be exfiltrated into the surrounding soil in areas with porous soils though exfiltration may not be permitted in Wellfield Zoning Districts.

Bioretention areas are designed for intermittent flow and to drain and aerate between rainfall events. Sites with continuous flow from groundwater, sump pumps or other areas must be avoided.

A schematic example of a bioretention area can be found in **Appendix E**, Post-Construction Stormwater Run Off Control Details.

Bioretention areas consist of:

- Grass filter strip between the contributing drainage area and the ponding area;
- Ponding areas containing vegetation with a planting soil bed;

- Organic/mulch layer; and
- Gravel and perforated pipe underdrain system to collect runoff that has filtered through the soil layers (bioretention areas can optionally be designed to infiltrate into the soil).

Optional design components include:

- Sand filter layer to spread flow, filter runoff and aid in aeration and drainage of the planting soil;
- Stone diaphragm at the beginning of the grass filter strip to reduce velocities and spread flow into the grass filter; and
- Inflow diversion or an overflow structure.

#### **7.2.4.1 Site and Design Considerations**

The following design and site considerations must be followed when designing bioretention areas:

- The drainage area (contributing or effective) must be 5 acres or less, though 0.5 to 2 acres is preferred;
- The minimum size for facility is 200 ft<sup>2</sup>, with a length to width ratio of 2:1. Slope of the site may not exceed 6%;
- Soil filter beds must be sized using a Darcy's Law equation with a filter bed drain time of 48 hours and a coefficient of permeability (k) of 0.5 ft/day. The soil bed must be at least 4 feet deep. Planting soils must be sandy loam, loamy sand or loam texture with a clay content rating from 10 to 25 percent. The soil must have an infiltration rate of at least 0.5 inches per hour and a pH between 5.5 and 6.5. In addition, the planting soil shall have a 1.5 to 3 percent organic content and a maximum 500-ppm concentration of soluble salts;
- The maximum ponding depth in bioretention areas is 6 inches;
- Filter strip design for pre-treatment must follow the requirements outlined in Section 7.2.7;

- The mulch layer must consist of 2-4 inches of commercially available fine shredded hardwood mulch or shredded hardwood chips;
- The sand bed must be 12-18 inches thick. Sand must be clean and have less than 15% silt or clay content;
- Pea gravel for the diaphragm and curtain, where used, must be ASTM D 448 size No. 6 (1/8" to 1/4");
- The underdrain collection system must be equipped with a 6 inch perforated PVC pipe in an 8-inch gravel layer. The pipe must have 3/8-inch perforations, spaced on 6-inch centers with a minimum of 4 holes per row. The pipe is spaced at a maximum of 10 feet on center, and a minimum grade of 0.5% must be maintained. A permeable filter fabric is placed between the gravel layer and the planting soil bed;
- The required elevation difference needed from the inflow to the outflow is 5 feet;
- The depth from the bottom of the bioretention facility to the seasonally high water table must be a minimum of 2 feet;
- Runoff captured by facility must be sheet flow to prevent erosion of the organic or mulch layer. Velocities entering the mulch layer must be between 1-2 fps;
- Continuous flow from groundwater, sump pumps or other areas to the bioretention area is prohibited;
- An overflow structure and a non-erosive overflow channel must be provided to safely pass the flow from the bioretention area that exceeds the storage capacity to a stabilized downstream area. The high flow structure within the bioretention area can consist of a yard drain catch basin, with the throat of the catch basin inlet typically 6 inches above the elevation of the shallow ponding area; and
- If the bioretention area is used as a sediment control measure during active construction, the sediment must be cleaned out of the bioretention area and elevations

and grades reestablished as noted in the approved stormwater management plan for post-construction runoff control.

#### **7.2.4.2 Performance Standards**

Bioretention areas designed, constructed and maintained as noted in this manual shall provide the following pollutant reductions:

<b>Pollutant</b>	<b>Percent Reduction</b>
TSS	80%
Total P	30%
Total N	50%
Metals	60%

#### **7.2.4.3 Advantages**

The advantages of bioretention areas are as follows:

- They are applicable to small drainage areas;
- They are often located in landscape islands;
- They have the capability for high pollutant removal; and
- There is greater community acceptance, if designed and maintained correctly.

#### **7.2.4.4 Disadvantages**

The disadvantages of bioretention areas include:

- They require extensive landscaping; and
- They are not recommended for areas with steep slopes.

#### **7.2.4.5 Maintenance**

Landscaping is critical to the performance and function of the bioretention area. A dense and vigorous groundcover must be established over the contributing pervious drainage area before runoff can be diverted into the facility.

The BMP owner is responsible for maintenance and inspections. Each BMP on a site must have an operations



and maintenance plan. The BMP owner must maintain and update the BMP operations and maintenance plan. At a minimum, the operations and maintenance plan must include, but is not limited to:

- Inspection, repair and replacement of all treatment components;
- The bioretention area shall be vegetated like a terrestrial forest ecosystem, with a mature tree canopy, subcanopy of understory trees, scrub layer and herbaceous ground cover. Three species of each tree and shrub type shall be planted;
- The tree-to-shrub ration shall be 2:1 to 3:1. On average, trees shall be spaced 8 feet apart. Plants shall be placed at regular intervals to replicate a natural forest. Woody vegetation shall not be planted at inflow locations.
- After the trees and shrubs are established, the ground cover and mulch shall be established; and
- Use native plants, selected based upon hardiness and hydric tolerance.

### **7.2.5 Sand Filters**

Sand filters are structural stormwater controls that temporarily store stormwater and pass it through a filter bed of sand. Most sand filter systems contain two chambers. The first chamber is a sedimentation chamber that removes floatables and heavy sediments. The second chamber is the filtration chamber, which removes additional pollutants by filtering the runoff through a sand bed. The filtered runoff is typically collected and returned to the conveyance system, though it can be partially or fully exfiltrated into the surrounding soil in areas with porous soils.

Sand filters are primarily designed as off-line structures for stormwater quality and typically need to be used in conjunction with another structural BMP to provide water quantity control.

A schematic example of sand filters and variations can be found in **Appendix E**, Post-Construction Stormwater Quality Details.

### 7.2.5.1 Site and Design Considerations

The following design and site considerations must be followed when designing sand filters:

- The maximum-effective drainage area to an individual stormwater filtering system is less than 10 acres. Sand filters cannot be designed to treat the entire contributing drainage area;
- The design volume must be based on one-inch rainfall and must be designed to fully empty in 36 hours;
- Adequate pretreatment (e.g., filter strips, see Section 7.2.7) is required to prevent sediment from overloading the filters. The inlet structure to the filtration chamber must be designed to spread the flow uniformly across the surface of the filter media. Stone riprap or other dissipation devices must be installed to prevent gouging of the sand media and to promote uniform flow;
- The allowable minimum head is one foot. The maximum allowable head is 6 feet;
- Construct sand bed to a depth of at least 18 inches;
- Underdrain pipes must consist of main collector pipes and perforated lateral branch pipes. Reinforce the underdrain piping to withstand the weight of the overburden. Internal diameters of lateral branch pipes must be 4 inches or greater (6 inches preferred) and perforations shall be 1/8 inch. Space perforations a maximum of 6 inches between rows. All piping must be schedule 40 polyvinyl chloride or greater strength or similarly rated HDPE pipe. The minimum grade of piping shall be 1/8 inch per foot (1% slope). Provide access for cleaning all underdrain piping;
- Surface filters may have a grass cover to aid in pollution adsorption;
- Establish vegetation over the contributing drainage areas before runoff can be accepted into the facility; and

- Two allowable surface sand bed filter configurations are:

*Sand Bed with Gravel Layer*

- Top layer of sand must be a minimum of 18 inches of 0.02 - 0.04 inch diameter sand (smaller sand size is acceptable).
- A layer of one-half to 2-inch diameter gravel under the sand must be provided for a minimum of 2 inches of cover over the top of the under-drain lateral pipes.
- No gravel is required under the lateral pipes.
- A layer of geotextile fabric (permeable filter fabric) must separate the sand and gravel.

*Sand Bed with Trench Design*

- Top layer of sand is to be 12-18 inches of 0.02 - 0.04 inch diameter sand (smaller size is acceptable).
- Laterals to be placed in trenches with a covering of one-half to 2- inch gravel and geotextile fabric.
- The lateral pipes are to be underlain by a layer of drainage matting.
- A presettling basin and/or biofiltration swale is recommended to pretreat runoff discharging to the sand filter.
- A maximum spacing of 10 feet between lateral underdrain pipes is recommended.

**7.2.5.2 Performance Standards**

Sand filters designed, constructed and maintained as noted in this manual shall provide the following pollutant reductions:

<b>Pollutant</b>	<b>Percent Reduction</b>
BOD	60%
TSS	85%
Total P	65%
Total N	50%
Bacteria	40-80%
Metals	60%

### 7.2.5.3 Variations

There are three primary sand filter system designs, the surface sand filter, the perimeter sand filter, and the underground sand filter.

**Surface Sand Filter-** The surface sand filter is a ground-level open-air structure that consists of a pretreatment sediment forebay and a filter bed chamber. This system can treat drainage areas up to 10 acres in size and is typically located off-line. Surface sand filters can be designed as an excavation with an earthen embankment or as a concrete structure.

**Perimeter Sand Filter-** The perimeter sand filter is an enclosed filter system typically constructed just below grade in a vault along the edge of an impervious area such as a parking lot. The system consists of a sedimentation chamber and a sand bed filter. Runoff flows into the structure through a series of inlet grates located along the top of the control.

**Underground Sand Filter-** The underground sand filter is intended primarily for extremely space-limited and high-density areas.

### 7.2.5.4 Advantages

The advantages of sand filters include:

- They are applicable to small drainage areas;
- They are good for highly impervious areas; and
- They have good retrofit capability.

### 7.2.5.5 Disadvantages

The disadvantages of sand filters include:

- They are high maintenance;
- They are not recommended for areas with high sediment content in stormwater;
- They can be relatively costly; and

- There are possible odor problems associated with the use of them.

#### **7.2.5.6 Maintenance**

The developer is responsible for the inspection and the maintenance of each sand filter. Each BMP must have an operations and maintenance plan. The BMP owner must maintain and update the BMP operations and maintenance plan, as necessary. The operations and maintenance plan must include, but is not limited to:

- The removal of sediment layer buildup during dry periods with steel rakes or other devices.
- The replacement of some or all of the sand when permeability of the filter media is reduced to unacceptable levels, which shall be specified in the design of the facility. A minimum infiltration rate of 0.5 inches per hour shall be used for all infiltration designs.

#### **7.2.6 Dry Water Quality Swales**

Dry water quality swales are channels designed and constructed to capture and treat stormwater runoff within dry cells formed by check dams or other means. Dry water quality swales are also described as biofiltration swales. These swales are designed with a limited slope for slow, shallow flow to allow particulates to settle out and to promote infiltration. Water quality swales are limited to areas with low impervious acreage, such as residential and industrial developments.

Dry swales are channels designed with a filter bed and underdrain system. They are designed to filter and infiltrate the entire WQv through the bottom of the swale. Runoff is collected by a perforated pipe and discharged at the outlet. Water quality swales are dry most of the time and are therefore well suited for residential areas.

A schematic example of a dry water quality swale can be found in **Appendix E**, Post-Construction Stormwater Run Off Control Details.

##### **7.2.6.1 Site and Design Considerations**

The following design and site considerations must be followed when designing water quality swales:

- Water quality swales treat only the WQv. An additional measure is needed to provide detention in conjunction with the water quality swale. The swales can be designed as on-line or off-line structures. Larger storms pass non-erosively through the channels;
- Water quality swales are limited to peak discharges generally less than 5 to 10 cfs and runoff velocities less than 2.5 ft/sec. The maximum drainage area is 5 acres. The maximum ponding time must be less than 48 hours, and a minimum ponding time of 30 minutes is recommended;
- The maximum design flow depth is 1 foot, with a ponding depth of 18 inches at the end of the channel;
- Swale cross-section must have side slopes of 3:1 (h:v) or flatter. Bottom widths must be between 2-8 feet wide;
- Underlying soils shall have a high permeability ( $f_c > 0.5$  inches per hour). Seasonally high water table must be greater than 3 feet below the bottom of the swale;
- Water quality swales must have a minimum length of 100 feet; and
- Provide a sediment forebay at the inlet to the swales.

#### 7.2.6.2 Performance Standards

Water quality swales designed, constructed and maintained (on a 4% or flatter slope) as noted in this manual shall provide the following pollutant reductions:

Pollutant	Percent Reduction
BOD	10%
TSS	80%
Total P	83%
Total N	92%
Metals	75%

#### 7.2.6.3 Advantages

The advantages of water quality swales include:

- They are typically well accepted in residential settings;
- They are inexpensive;
- They combine water quality treatment with runoff conveyance;
- They reduce runoff velocities; and
- They are low maintenance.

#### **7.2.6.4 Disadvantages**

The disadvantages of water quality swales include:

- They cannot be used on steep slopes; and
- They only provide a limited amount of stormwater quantity control.

#### **7.2.6.5 Maintenance**

The developer is responsible for the inspection and maintenance of each water quality swale. Each BMP must have an operations and maintenance plan. The BMP owner must maintain and update the BMP operations and maintenance plan, as necessary. The operations and maintenance plan must include, but is not limited to:

- Adequate access for inspection and maintenance.
- The maintenance of dry swales to keep grass cover dense and vigorous.
- At a minimum, maintenance shall include periodic mowing, occasional spot reseeding, and weed control. Swale grasses must never be mowed close to the ground. Grass heights in the 4 to 6 inch range are recommended.
- The fertilization of grass swale shall be done when needed to maintain the health of the grass, with care not to over-apply the fertilizer.
- The removal of sediment accumulated in forebay when it is 50% full.

## 7.2.7 Biofilters

Biofilters are densely vegetated sections of land, designed to treat runoff from and remove pollutants through vegetative filtering and infiltration. Biofilters must receive runoff from adjacent areas as sheet flow. The vegetation slows the runoff and filters out sediment and other pollutants. However, the TSS removal provided is less than 80 percent. Therefore, biofilters must be used in a treatment system in conjunction with other management practices to provide the 80 percent performance goal.

Biofilters are best suited to treating runoff from roadways, rooftops, small parking areas and pervious areas. They can be easily incorporated into residential development as land-use buffers and setbacks.

A schematic example of biofilters can be found in **Appendix E**, Post-Construction Stormwater Quality Details.

### 7.2.7.1 Site and Design Considerations

The following site and drainage considerations must be followed when designing biofilters:

- To ensure sheet flow into the filter strips and riparian buffers, flow spreaders or level spreaders must be designed and installed where concentrated runoff flows into filter strips or riparian buffers;
- Level Spreader: The grade of a level spreader shall be 0%. The channel grade for the last 20 feet of the dike or diversion entering the level spreader must be less than or equal to 1% and designed to provide a smooth transition into spreader. The depth of a level spreader as measured from the lip must be at least 6 inches. The level spreader lip must be constructed on undisturbed soil (not fill material) to uniform height and zero grade over length of the spreader. The maximum drainage area to the level spreader shall be 10 acres or less with the optimal size being less than 5 acres. The maximum flow into the level spreader must be 30 cfs or less;



- Appropriate length, width, and depth of level spreaders shall be selected from the following table;

Design Flow (cfs)	Entrance Width (ft)	Depth (ft)	End Width (ft)	Length (ft)
0-10	10	0.5	3	10
10-20	16	0.6	3	20
20-30	24	0.7	3	30

- Capacity of the spreader, filter strip and riparian buffer length (perpendicular to flow) must be determined by estimating the volume of flow that is diverted to the spreader for water quality control;
- The released runoff to the outlet must be on undisturbed stabilized areas in sheet flow and not allowed to re-concentrate below the structure;
- Slope of the filter strip from a level spreader must not exceed 10 percent;
- All disturbed areas must be vegetated immediately after construction;
- The minimum filter strip width is 20 feet;
- Filter strips must be designed for slopes between 2 percent and 6 percent;
- Ensure that flows in excess of design flow move across and around the filter strip without damaging it;
- Filter strips can be used effectively as pretreatment measures. The minimum sizing criteria are as follows:

Parameters	Impervious Area				Pervious Area			
	Maximum inflow Approach length (ft)	35		75		75		100
Filter strip slope (max = 6%)	<2%	>2%	<2%	>2%	<2%	>2%	<2%	>2%
Filter strip minimum length	10	15	20	25	10	12	15	18

- Riparian buffers: The use of buffers is limited to drainage areas of 10 acres or less with the optimal size being less than 5 acres;
- Slope of the buffer from a level spreader cannot exceed 10 percent; and
- The top edge of buffer must directly abut the contributing impervious area and follow the same elevation contour line.

### 7.2.7.2 Performance Standards

Biofilters designed, constructed and maintained as noted in this manual shall provide the following pollutant reductions:

Pollutant	Percent Reduction (riparian buffer/filter strip)
BOD	40/10%
TSS	60/30%
Total P	35/10%
Total N	25/10%
Metals	70/30%

### 7.2.7.3 Variations

**Filter strip:** A filter strip is a uniformly graded and densely vegetated strip of land. The vegetation can be grasses or a combination of grass and woody plants. Pollutant removal efficiencies are based upon a 50-foot wide strip. Uniform sheet flow must be maintained through the filter strip to provide pollutant reduction and to avoid erosion.

**Riparian buffer:** A riparian buffer is a strip of land with natural, woody vegetation along a stream or other

watercourse. Besides the undergrowth of grasses and herbaceous vegetation, the riparian buffer includes deep rooted trees. The 20-foot zone closest to the stream or watercourse contains the trees, while the outer 30 feet of the riparian buffer contains a dense stand of grasses. The overall width of the riparian buffer is 50 feet. Uniform sheet flow must be maintained through the filter strip to provide pollutant reduction and to avoid erosion.

#### **7.2.7.4 Advantages**

The advantages of biofilters include:

- Filter strips and riparian buffers can easily be incorporated into new development design;
- They are low maintenance once a dense ground cover is established in filter strips and level spreaders and once trees and other woody vegetation is established in riparian buffers; and
- Riparian buffers provide wildlife habitat.

#### **7.2.7.5 Disadvantages**

The disadvantages of biofilters include:

- Filter strips, riparian buffers and level spreaders have limited drainage areas; and
- Constructing a level lip on a level spreader can be difficult. Failure to construct a level lip makes the level spreader ineffective.

#### **7.2.7.6 Maintenance**

The developer is responsible for the inspection and maintenance of all biofilters. Each BMP must have an operations and maintenance plan. The BMP owner must maintain and update the BMP operations and maintenance plan, as necessary. The operations and maintenance plan must include, but is not limited to:

- The mowing of the biofilter as necessary, or the utilization of an appropriate management technique for existing species.

- The removal of sediment accumulation and debris in order to sustain sediment removal efficiency.

### **7.2.8 Catch Basin Inserts**

Many variations of catch basin insert designs exist. Catch basin inserts can be designed and installed in a storm drain system provided the following minimum criteria for the inserts are met:

- Provide an overflow weir to pass storm events larger than the design storm.
- Catch basin inserts must meet the 80% TSS removal rate.
- Each design for catch basins can have specific maintenance needs or issues. Maintenance requirements must be clearly defined, and a specific maintenance agreement submitted to the City for review and approval.

Supporting documentation from the manufacturer to verify maintenance requirements and TSS removal rates must be submitted to the City for verification and approval. A maintenance plan must be submitted to the City prior to stormwater management plan approval and maintained and updated by the BMP owner. The BMP owner is responsible for routine maintenance, operation and inspection.

### **7.2.9 Other Pre-Approved BMPs**

Other pre-approved BMPs include, but are not limited to, porous pavement systems, gravity oil grit separators, equipment maintenance and washing areas, infiltration trenches, hydrodynamic separators, hazardous material storage, subsurface detention, and parking lot islands.

The applicant should consult *The Indiana Stormwater Quality Manual* (formerly *The Indiana Handbook for Erosion Control in Developing Areas*) for detailed design, construction and maintenance criteria for these post-construction stormwater management practices.

**CITY OF HUNTINGTON  
STORMWATER DEVELOPMENT MANUAL**

**SUBMITTAL PACKAGE CHECKLIST FOR  
CATEGORY 1 PROJECTS**

(Definition of Category 1: Land disturbing activities greater than one acre.)

Project Name: \_\_\_\_\_

Project Location and Description: \_\_\_\_\_

Owner Name: \_\_\_\_\_

Address: \_\_\_\_\_

Telephone Number: \_\_\_\_\_

Name of Engineer: \_\_\_\_\_

Address: \_\_\_\_\_

Telephone Number: \_\_\_\_\_

	For City Use Only			Project Narrative
	Submitted by Applicant	Adequate	Deficient	
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Scope and Purpose of Project</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Legal Description</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Existing Soil Conditions including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Properties
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Characteristics
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Limitations
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Hazards
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Measures utilized to overcome/ minimize adverse soil conditions
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Construction Sequence including Phases</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>14-Digit Hydrologic Unit Code</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Project Site Map or Plat including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Lot numbers
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Lot boundaries
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Road layout
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Road names
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Right-of-way
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Identification of Other State/ Federal Water Quality Permits Required</b>

Submitted by Applicant	Adequate	Deficient	Not Applicable	Vicinity Map
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Depicts Project Site Location in Relation to Landmarks, Towns, Major Roads

Submitted by Applicant	Adequate	Deficient	Not Applicable	General Plan Requirements
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	North arrow
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Graphic and written scale
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Legend for all symbols
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Location and elevation of property benchmark
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Professional seal and signature
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Date
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Table of revisions with dates (when applicable)

Submitted by Applicant	Adequate	Deficient	Not Applicable	Existing Project Site Layout
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Location and name of wetlands, lakes, water courses on or adjacent to site
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Location of existing structures on site
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Existing stormwater drainage systems including:
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Location
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Size
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Dimension
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Slope
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Flow direction
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	100-year floodplains, floodway fringes, floodways (note if none exists)
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Soil map of predominant soil types
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Identification and delineation of vegetative cover
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Land use of adjacent properties
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Existing topography at a contour interval (2' min.) to indicate drainage pattern
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Adjacent site topography (when applicable)

				<b>Final Project Site Layout</b>
Submitted by Applicant	Adequate	Deficient	Not Applicable	
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Location of all proposed site improvements including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Roads
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Utilities
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Lot delineation and identification
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Proposed structures
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Common areas
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>100-year floodplains, floodway fringes, floodways (note if none exists)</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Proposed topography at a contour interval (2' min.) to indicate drainage pattern</b>

				<b>Grading Plan</b>
Submitted by Applicant	Adequate	Deficient	Not Applicable	
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Delineate proposed land disturbing activities, including those off-site</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Locate soil stockpiles and borrow areas</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Information regarding off-site borrow, stockpile, disposal areas that are under the control of the project site owner</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Existing and proposed topographic information</b>

				<b>Drainage Plan</b>
Submitted by Applicant	Adequate	Deficient	Not Applicable	
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Peak discharge of 10-year storm event including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Preconstruction conditions
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Post construction conditions
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Location, size, dimension, slope, direction of flow of stormwater drainage systems shown on plan and profile views</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Locations where stormwater may be directly discharged into ground water, note if none exist</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Locations of points where stormwater discharge will leave the site</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Name of receiving waters</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Location, size, and dimensions of features used for stormwater management</b>

Submitted by Applicant	Adequate	Deficient	Not Applicable	<b>Erosion and Sediment Control Plan</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Location, dimensions, detailed specifications, construction details of all temporary and permanent stormwater quality measure</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Temporary and Permanent Stabilization Plans including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Sequence of implementation
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Specifications and application rates for soil amendments and seed mixtures
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Type and application rate for anchored mulch
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Construction sequence describing relationship between implementation of stormwater quality measures and stages of construction activities</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Self-monitoring program including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Plan
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Procedures
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Maintenance plan of measures
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Description of potential pollutant sources associated with construction activities that may reasonably be expected to add a significant amount of pollutants to stormwater discharges</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Material handling and storage associated with construction activity shall meet the spill prevention and spill response requirements in 327 IAC 2-6.1</b>

Submitted by Applicant	Adequate	Deficient	Not Applicable	<b>Post-Construction Stormwater Management Plan</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Description of potential pollutant sources from proposed land use that may reasonably be expected to add a significant amount of pollutants to stormwater discharges</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Location, dimensions, detailed specifications, and construction details of all post construction stormwater quality measures.</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Description of measures that will be installed to control pollutants in stormwater discharges after completion of construction activities.</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Sequence of installation of measures</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Measures to remove or minimize pollutants from stormwater run-off</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Measures to prevent or minimize adverse impacts to stream and riparian habitat</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Description of maintenance guidelines for measures including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Facilitates proper long-term function
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Availability to future parties whom will be responsible for operation and maintenance of measures



Submitted by Applicant	Adequate	Deficient	Not Applicable	<b>Calculations</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Existing and Proposed Estimation of Stormwater Runoff Calculations including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Drainage area map (note all subareas) with all acreage and runoff coefficients and entering off-site drainage noted
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Weighted runoff coefficient computations
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Time of concentration computations
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Closed Conduit and Open Channel Design Calculations including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Size of pipe or channel cross-section
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Pipe or channel's slope w/invert and inlet elevations
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Roughness coefficient
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Pipe length
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Velocities
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Capacities
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Pipe type and class
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Design runoff of component sheds
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Inlet spacing calculations
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Off-Site Drainage and Downstream Capacity Calculations (if applicable)</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Detention/Retention Pond Calculations</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Pre-developed shed map
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Post-developed shed map
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Pre- and post-developed time of concentrations calculations
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Overland flow route to detention/retention pond
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Peak design discharge
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Time to peak discharge
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Total design storm runoff
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Inflow hydrograph
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Spillway peak outflow rate
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Emergency spillway capacity calculations
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Off-site bypass flow route
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Bottom pond slope
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Flood protection grades on plat

Submitted by Applicant	Adequate	Deficient	Not Applicable	<b>Miscellaneous</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Submittal Package Checklist</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b># Full-Sized Sets of Plans</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b># Half-Sized Sets of Plans</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Permit Filing Fee of \$50</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Petition for Regulated Drain</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Electronic AutoCAD Files of Complete Set of Plans (if available)</b>

This checklist is not intended to be comprehensive. The Stormwater Design Criteria Ordinance and the Ordinance for Erosion and Sediment Control, along with other sections of the Stormwater Development Manual, shall be referenced to assure all required elements are integrated into the submittal package.

**NOTES:**

---



---



---



---



---



---



---



---

**TO BE COMPLETED BY THE CITY OF HUNTINGTON  
City Engineering Department**

DATE OF REJECTION LETTER SENT \_\_\_\_\_ ACCEPTANCE \_\_\_\_\_

ACCEPTED FOR REVIEW BY: \_\_\_\_\_

FIRM ASSIGNED TO REVIEW: \_\_\_\_\_

DATE OF REVIEW INITIATION: \_\_\_\_\_ REVIEWER \_\_\_\_\_

DATE THE REVIEW COMMENCED: \_\_\_\_\_

**CITY OF HUNTINGTON  
STORMWATER DEVELOPMENT MANUAL**

**SUBMITTAL PACKAGE CHECKLIST FOR  
CATEGORY 2 PROJECTS**

*(Definition of Category 2: Single family residential development consisting of four or fewer lots or a single-family residential strip development where the developer offers for sale or lease without land improvements, and the project is not part of a larger common plan of development or sale.)*

Project Name: \_\_\_\_\_

Project Location and Description: \_\_\_\_\_

Owner Name: \_\_\_\_\_

Address: \_\_\_\_\_

Telephone Number: \_\_\_\_\_

Name of Engineer: \_\_\_\_\_

Address: \_\_\_\_\_

Telephone Number: \_\_\_\_\_

	For City Use Only			Project Narrative
	Submitted by Applicant	Adequate	Deficient	
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Scope and Purpose of Project</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Legal Description</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Existing Soil Conditions including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Properties
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Characteristics
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Limitations
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Hazards
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Measures utilized to overcome/ minimize adverse soil conditions
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Construction Sequence including Phases</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>14-Digit Hydrologic Unit Code</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Project Site Map or Plat including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Lot numbers
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Lot boundaries
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Road layout
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Road names
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Right-of-way

<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Identification of Other State/ Federal Water Quality Permits Required</b>
Submitted by Applicant	Adequate	Deficient	Not Applicable	<b>Vicinity Map</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Depicts Project Site Location in Relation to Landmarks, Towns, Major Roads</b>

Submitted by Applicant	Adequate	Deficient	Not Applicable	<b>General Plan Requirements</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>North arrow</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Graphic and written scale</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Legend for all symbols</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Location and elevation of property benchmark</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Professional seal and signature</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Date</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Table of revisions with dates (when applicable)</b>

Submitted by Applicant	Adequate	Deficient	Not Applicable	<b>Existing Project Site Layout</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Location and name of wetlands, lakes, water courses on or adjacent to site</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Location of existing structures on site</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Existing stormwater drainage systems including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Location
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Size
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Dimension
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Slope
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Flow direction
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>100-year floodplains, floodway fringes, floodways (note if none exists)</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Soil map of predominant soil types</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Identification and delineation of vegetative cover</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Land use of adjacent properties</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Existing topography at a contour interval (2' min.) to indicate drainage pattern</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Adjacent site topography (when applicable)</b>

<b>Final Project Site Layout</b>				
Submitted by Applicant	Adequate	Deficient	Not Applicable	
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Location of all proposed site improvements including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Roads
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Utilities
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Lot delineation and identification
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Proposed structures
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Common areas
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>100-year floodplains, floodway fringes, floodways (note if none exists)</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Proposed topography at a contour interval (2' min.) to indicate drainage pattern</b>

<b>Grading Plan</b>				
Submitted by Applicant	Adequate	Deficient	Not Applicable	
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Delineate proposed land disturbing activities, including those off-site</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Locate soil stockpiles and borrow areas</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Information regarding off-site borrow, stockpile, disposal areas that are under the control of the project site owner</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Existing and proposed topographic information</b>

<b>Drainage Plan</b>				
Submitted by Applicant	Adequate	Deficient	Not Applicable	
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Peak discharge of 10-year storm event including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Preconstruction conditions
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Post construction conditions
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Location, size, dimension, slope, direction of flow of stormwater drainage systems shown on plan and profile views</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Locations where stormwater may be directly discharged into ground water, note if none exist</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Locations of points where stormwater discharge will leave the site</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Name of receiving waters</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Location, size, and dimensions of features used for stormwater management</b>

Submitted by Applicant	Adequate	Deficient	Not Applicable	<b>Erosion and Sediment Control Plan</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Location, dimensions, detailed specifications, construction details of all temporary and permanent stormwater quality measure</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Temporary and Permanent Stabilization Plans including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Sequence of implementation
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Specifications and application rates for soil amendments and seed mixtures
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Type and application rate for anchored mulch
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Construction sequence describing relationship between implementation of stormwater quality measures and stages of construction activities</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Self-monitoring program including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Plan
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Procedures
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Maintenance plan of measures
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Description of potential pollutant sources associated with construction activities that may reasonably be expected to add a significant amount of pollutants to stormwater discharges</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Material handling and storage associated with construction activity shall meet the spill prevention and spill response requirements in 327 IAC 2-6.1</b>

Submitted by Applicant	Adequate	Deficient	Not Applicable	Calculations
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Existing and Proposed Estimation of Stormwater Runoff Calculations including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Drainage area map (note all subareas) with all acreage and runoff coefficients and entering off-site drainage noted
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Weighted runoff coefficient computations
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Time of concentration computations
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Closed Conduit and Open Channel Design Calculations including:</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Size of pipe or channel cross-section
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Pipe or channel's slope w/invert and inlet elevations
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Roughness coefficient
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Pipe length
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Velocities
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Capacities
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Pipe type and class
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Design runoff of component sheds
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Inlet spacing calculations
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Off-Site Drainage and Downstream Capacity Calculations (if applicable)</b>

Submitted by Applicant	Adequate	Deficient	Not Applicable	Miscellaneous
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Submittal Package Checklist</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b># Full-Sized Sets of Plans</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b># Half-Sized Sets of Plans</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Permit Filing Fee of \$50</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Petition for Regulated Drain</b>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<b>Electronic AutoCAD Files of Complete Set of Plans (if available)</b>

This checklist is not intended to be comprehensive. The Stormwater Design Criteria Ordinance and the Ordinance for Erosion and Sediment Control, along with other sections of the Stormwater Development Manual, shall be referenced to assure all required elements are integrated into the submittal package.

NOTES:

---

---

---

---

---

---

---

---

---

---

**TO BE COMPLETED BY THE CITY OF HUNTINGTON**  
**City Engineering Department**

DATE OF REJECTION LETTER SENT \_\_\_\_\_ ACCEPTANCE \_\_\_\_\_

ACCEPTED FOR REVIEW BY: \_\_\_\_\_

FIRM ASSIGNED TO REVIEW: \_\_\_\_\_

DATE OF REVIEW INITIATION: \_\_\_\_\_ REVIEWER \_\_\_\_\_

DATE THE REVIEW COMMENCED: \_\_\_\_\_



**CITY OF HUNTINGTON  
STORMWATER DEVELOPMENT MANUAL  
SUBMITTAL PACKAGE CHECKLIST FOR  
CATEGORY 3 PROJECTS**

*(Definition of Category 3: Land disturbing activities for individual lots of any size within a permitted project that is over one acre.)*

Name of Permitted Project: \_\_\_\_\_

Lot Name / Lot Number: \_\_\_\_\_

Lot Name / Lot Number Location and Description: \_\_\_\_\_

Owner of Permitted Project's Name: \_\_\_\_\_

Address: \_\_\_\_\_

Telephone Number: \_\_\_\_\_

Name of Lot Operator: \_\_\_\_\_

Address: \_\_\_\_\_

Telephone Number: \_\_\_\_\_

(See General Requirements on Page 2)

Submitted by Applicant or Agreed to by Applicant	For City Use Only		<b>General Requirements</b>
	Adequate	Deficient	
<input type="checkbox"/>			<b>Stormwater Pollution Prevention Plan from Permitted Project</b>
<input type="checkbox"/>			<b>General Requirements of Lot Plan</b>
<input type="checkbox"/>			General layout of lot and position of building
<input type="checkbox"/>			North arrow
<input type="checkbox"/>			Graphic and written scale
<input type="checkbox"/>			Date
<input type="checkbox"/>			Stable construction site access
<input type="checkbox"/>			Appropriate perimeter erosion and sediment control
<input type="checkbox"/>			<b>Lot Operator Agrees to the Following</b>
<input type="checkbox"/>			Sediment discharge and tracking from lot shall be minimized throughout the land disturbing activities until permanent stabilization is achieved
<input type="checkbox"/>			Sediment that is tracked or washed shall be redistributed or disposed of in a manner that is in compliance with all applicable statutes and rule. Sediment shall not be flushed from the streets with water.
<input type="checkbox"/>			Adjacent lots disturbed by the lot operator shall be stabilized with temporary or permanent surface
<input type="checkbox"/>			Final stabilization shall be installed before permit termination
<input type="checkbox"/>			Notify the homeowner / lot owner of the requirement for, and benefits of, final stabilization

Printed Name of Lot Operator: \_\_\_\_\_

Signature of Lot Operator: \_\_\_\_\_ Date: \_\_\_\_\_

This checklist is not intended to be comprehensive. The Stormwater Design Criteria Ordinance and the Ordinance for Erosion and Sediment Control, along with other sections of the Stormwater Development Manual, shall be referenced to assure all required elements are integrated into the submittal package.

NOTES:

---

---

---

---

---

---

---

---

---

---

**TO BE COMPLETED BY THE CITY OF HUNTINGTON  
CITY ENGINEERING DEPARTMENT**

DATE OF REJECTION LETTER SENT \_\_\_\_\_ ACCEPTANCE \_\_\_\_\_

ACCEPTED FOR REVIEW BY: \_\_\_\_\_

CITY DEPARTMENT / FIRM ASSIGNED TO REVIEW: \_\_\_\_\_

DATE OF REVIEW INITIATION: \_\_\_\_\_ REVIEWER: \_\_\_\_\_

DATE THE REVIEW COMMENCED: \_\_\_\_\_

**CITY OF HUNTINGTON  
STORMWATER DEVELOPMENT MANUAL**

**STORMWATER PERMIT APPLICATION**

GENERAL INFORMATION:

Project Name: \_\_\_\_\_

Project Location or Lot Location and Description: \_\_\_\_\_

Owners \_\_\_\_\_ Name: \_\_\_\_\_

Address: \_\_\_\_\_

Telephone Number: \_\_\_\_\_

Name of Engineer or Lot Operator: \_\_\_\_\_

Address: \_\_\_\_\_

Telephone Number: \_\_\_\_\_

Check one of the following:

- Category 1: The project is a land-disturbing activity covering one or more acres of land but does not fall under any of the following categories.
- Category 2: The project is a land-disturbing activity covering one or more acres of land and is a single family residential development consisting of four (4) or fewer lots or a single-family strip development where the developer offers for sale or lease without land improvements, and the project is not part of a larger common plan of development or sale.
- Category 3: The project is an individual lot of any size within a permitted project.

APPLICATION STATEMENT: All required information and attachments required as part of the Submittal Package Checklist are included with this permit application. All required information and attachments required as a part of the Submittal Package Checklist are, to my knowledge and belief both true and accurate.

Owner, Engineer, or Lot Operators Printed Name: \_\_\_\_\_

Owner, Engineer, or Lot Operators Signature: \_\_\_\_\_ DATE \_\_\_\_\_

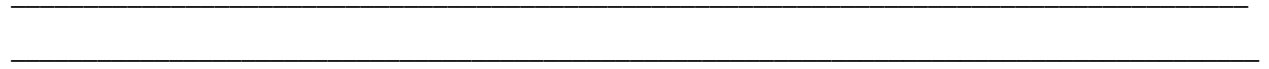
\*\*\*\*\* TO BE COMPLETED BY  
ADMINISTRATOR\*\*\*\*\*

Are the submittals in proper form and sufficient to permit reasonably thorough review? Yes / No

By (printed name): \_\_\_\_\_ Date: \_\_\_\_\_

By (signature): \_\_\_\_\_

Reason(s) if Not Accepted: \_\_\_\_\_



## Construction Site Soil Erosion Inspection Log

Project Name: \_\_\_\_\_

Project Location and Description: \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

Date: \_\_\_\_\_

Time: \_\_\_\_\_

Reason for Inspection:

- Complaint
- Weekly Inspection
- Inspection After Storm Water Runoff; Rain Gauge Reading \_\_\_\_\_
- Other \_\_\_\_\_

Observations: \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

Corrective Actions Taken or Needed: \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

Inspected By: \_\_\_\_\_

Signature: \_\_\_\_\_

**AGREEMENT FOR THE CONSTRUCTION, MAINTENANCE, AND REPAIR OF A  
STORMWATER MANAGEMENT FACILITY**

**WHEREAS**, the undersigned is/are the owner(s) (hereinafter "Owners") of certain real estate located in Huntington County, Indiana, and legally described herein as Exhibit "A," and

**WHEREAS**, the Owners are desirous of constructing a stormwater management facility over a portion of said real estate described at Exhibit "A;" and

**WHEREAS**, in order to construct such stormwater detention the Owners must obtain review and permits from the City of Huntington (hereinafter "City").

**NOW THEREFORE**, for good and valuable consideration, receipt of which is herein acknowledged, the Owner covenants with the City as follows:

1. Owner agrees to obtain prior approval from the City for design and construction of the stormwater management facility. Plans and specifications, which Owner shall strictly adhere to, will be on file with the Development Services Department of the City of Huntington, Indiana. Failure to obtain said approval will render this Agreement void and any stormwater management facility on this property illegal.
2. Owner agrees to construct and be responsible for the perpetual, maintenance, repair, and replacement, if necessary, of the stormwater management facility that will serve the real estate identified in Exhibit "A." Owner agrees to maintain said facility in a condition acceptable to the City.
  - a. Maintenance shall include, but not be limited to cosmetic maintenance (including structural) to the stormwater management facility during and after construction, mowing, weed control, trash pick-up, algae, mosquito control, and general maintenance.
  - b. All maintenance will be done so as to assure that all stormwater runoff designed to be detained within the stormwater management facility will be so detained, and the designed rate of runoff will not be increased after the improvements have been constructed as contemplated upon the said real estate.
  - c. Owner expressly grants the right of entry over, across, and through the real estate described herein at Exhibit "A" to the City for the purpose of inspecting, evaluating, or repairing the stormwater management facility.
  - d. City will notify the Owner in writing of maintenance or repairs required. Said notice may be delivered by U.S. mail to the last provided address of the Owner. If the Owner fails to address the problems outlined satisfactorily within thirty (30) days of said notice, City shall have the right to enter over and upon all necessary portions of the real estate as described in Exhibit "A" for the purpose of repairing and maintaining said stormwater management facility.
  - e. In the event that an emergency situation exists, as defined by the Board of Public Works of the City of Huntington, and the City is unable to locate the Owner or its agent after reasonable attempts have been made to do so, the City shall have the right to enter upon the real estate and make such emergency repairs as it deems necessary.
  - f. At any time the City is required to enter upon the real estate of the Owner and make such repairs, the costs of same shall be the obligation of the Owner. If the

Owner does not pay said costs within thirty (30) days after receiving written notice of the same from the City, the City shall have the right to place a lien against and upon the real estate (described in Exhibit "A") for costs incurred by it for the repair and maintenance of the stormwater management facility, including interest at the rate of eight (8%) percent per annum, and reasonable attorney's fees.

3. The Owner agrees to indemnify and hold harmless the City from all claims arising out of the installation, maintenance, or use of the stormwater management facility.
4. The most recent deed of record for the real estate has been recorded in the Office of the Recorder of Huntington County, Indiana, as document number\_\_\_\_\_.
5. This Agreement may only be amended by prior written consent signed by the Owner and the City.
6. The rights and obligations created by this Agreement shall be binding upon and shall run with the real estate for the benefit of the heirs, personal representative, successors and assigns of the Owner.
7. The laws of the State of Indiana shall govern this Agreement.

**IN WITNESS WHEREOF**, the parties hereto have executed this Agreement the day and year first above written.

Date:\_\_\_\_\_ Owner:\_\_\_\_\_

Printed Name:\_\_\_\_\_

Owner:\_\_\_\_\_

Printed Name:\_\_\_\_\_



STATE OF INDIANA        )  
                                  SS:    )  
COUNTY OF HUNTINGTON        )

**BEFORE ME**, a Notary Public, in and for said County and State, this \_\_\_\_ day of \_\_\_\_\_, 200\_\_ personally appeared the within named \_\_\_\_\_ and \_\_\_\_\_ who being first duly sworn upon their oath state that they are Owners of the subject real estate at Exhibit A and as such duly authorized to execute the foregoing instrument and acknowledged the same as their voluntary act and deed for the uses and purposes therein set forth.

**IN WITNESS WHEREOF**, hereunto subscribed my name, affixed my official seal.

\_\_\_\_\_  
**Notary Public**

\_\_\_\_\_  
**Printed Name of Notary**

**My Commission Expires:**

**Resident of \_\_\_\_\_ County**

**For the CITY OF HUNTINGTON**

**BY:** \_\_\_\_\_ Printed Name \_\_\_\_\_

**STATE OF INDIANA        )**  
                                  **SS:        )**  
**COUNTY OF HUNTINGTON        )**

**BEFORE ME**, a Notary Public, in and for said County and State, this \_\_\_\_\_day of \_\_\_\_\_, personally appeared the within named, \_\_\_\_\_, by me personally known, who being by me duly sworn has authority so to do and acknowledges said instrument to be the voluntary act and deed of said **City** for the uses and purposes therein set forth.

**IN WITNESS WHEREOF**, hereunto subscribed my name, affixed my official seal.

\_\_\_\_\_  
**Notary Public**

\_\_\_\_\_  
**Printed Name of Notary**

**My Commission Expires:**  
**Resident of \_\_\_\_\_ County**

**CITY OF HUNTINGTON  
STORMWATER DEVELOPMENT MANUAL  
PERMIT APPROVAL**

**STORMWATER CONSTRUCTION PERMIT**

This permit has been issued for the following project. The project has been reviewed and complies with the requirements described in the Huntington Stormwater Development Manual.

Any other City, State, and local permits that are needed for the execution of the project are the responsibility of the applicant or applicant's representative.

Any changes to the original submission will require re-permitting by the Stormwater Management Board and is the responsibility of the permit holder.

A copy of this Stormwater Construction Permit will be forwarded to the Huntington Building Department.

**GENERAL PROJECT INFORMATION:**

Name of proposed improvements project: \_\_\_\_\_

Location of Proposed improvements:

Along Road/Street \_\_\_\_\_ at or Between Roads/Streets \_\_\_\_\_

Project Owner: \_\_\_\_\_

Project Authorized Representative: \_\_\_\_\_

Mailing Address \_\_\_\_\_

Phone \_\_\_\_\_ Fax \_\_\_\_\_

\*\*\*\*\*

**TO BE COMPLETED BY ADMINISTRATOR**

The above project has been reviewed and approved for construction subject to the requirements included in the Huntington Stormwater Development Manual.

**DISPOSITION:**

Project Approved By: \_\_\_\_\_

Date: \_\_\_\_\_

\_\_\_\_\_  
Signature

**CITY OF HUNTINGTON  
STORMWATER DEVELOPMENT MANUAL**

**SUBMITTAL RESPONSE**

Date: \_\_\_\_\_

Project: \_\_\_\_\_

Owner: \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

Engineer: \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

Your submittal package

(     )     ***Has been accepted for review***

Firm assigned to the review: \_\_\_\_\_

Reviewer assigned to project: \_\_\_\_\_

Date of review initiation: \_\_\_\_\_

Comments can be expected (date): \_\_\_\_\_

(     )     ***Is in need of additional information before the review process can be initiated.***

These items were found to be omitted or did not correspond to the referenced page number on the submittal checklist:

\_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

Your cooperation and quick response will aid in an earlier review initiation.

## Post-Construction Stormwater Control Inspection Log

Project Name: \_\_\_\_\_

Project Location and Description: \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

Date: \_\_\_\_\_

Time: \_\_\_\_\_

Reason for Inspection:

- Complaint
- Yearly Inspection
- Inspection After Storm Water Runoff; Rain Gauge Reading \_\_\_\_\_
- Other \_\_\_\_\_

Observations: \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

Corrective Actions Taken or Needed: \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

Inspected By: \_\_\_\_\_

Signature: \_\_\_\_\_